Appendix F1 Preliminary Hydrology Study

PRELIMINARY HYDROLOGY STUDY TTM 82390 1515 WEST 178TH STREET GARDENA, CA

Project Address:

1515 West 178TH Street Gardena, CA 90248

Prepared For:

Melia Homes 8951 Research Drive Ste#100 Irvine, CA 92618 (949) 759-4367

Prepared By:

C&V Consulting Inc. 6 Orchard, Suite 200 Lake Forest, CA 92630 Contact: Dane McDougall, P.E. (949) 916-3800

> November 2018 Revised May 2019

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LACDPW Hydrology Map GIS Viewer for 50-yr 24hr Rainfall Depth LACPDW Hydrology Map GIS Viewer for Soil Type Los Angeles County Hydrology Manual 2006 – Appendix D

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Preliminary Grading Plan prepared by C&V Consulting, Inc. dated November 2018 Preliminary Utility Plan prepared by C&V Consulting, Inc. dated November 2018

APPENDIX E: As-Built

LACFCD 48" RCP BI 0432 – Line C As-built Plan

Preliminary Hydrology Study for TTM 82390, Gardena

This Preliminary Hydrology Study was prepared by C&V Consulting, Inc. under the supervision of Dane P. McDougall, P.E.

Dane P. McDougall, R.C.E. 80705 Principal, C&V Consulting, Inc.

Date

1.0 SITE DESCRIPTION:

The proposed site is located at 1515 West 178TH Street, in the City of Gardena, County of Los Angeles. The site is bordered by West 178TH Street to the south, existing commercial to the east, existing mobile home facility to the west, and existing vacant dirt lot & horse stables to the north. The existing site is approximately 5.63 acres and currently being utilized as a commercial facility that provides freight shipping services.

2.0 PURPOSE OF STUDY:

The preliminary hydrology study will determine the amount of stormwater runoff generated from the project site in the existing and proposed conditions. This study will anticipate whether detention basin will be required or not by comparing the proposed and existing condition peak flow rate for the 2, 25, 50 and 100-year peak storm events.

3.0 EXISTING CONDITIONS:

In the current condition, the northerly half of the site generally sheet flows over land towards the northwest corner of the site. Stormwater runoff enters an existing onsite storm drain inlet. The southerly half of the site generally sheet flows over land towards southwesterly corner of the site towards West 178th Street. Stormwater runoff tirubary to West 178th Street is conveyed as street flow within the existing curb/ gutter in the westerly direction and enters an existing LACFCD catch basin located about 120 feet west of the site. Stormwater runoff enters this existing catch basin, continues within an existing LACFCD 48" Reinforced Concrete Pipe (RCP) BI 0432 – Line C, and ultimately discharges into the existing Dominguez Channel (Refer to the As-built Drawing in the Appendix E for reference). Stormwater runoff tributary to the existing onsite storm drain inlet directly connects the existing LACFCD 48" storm drain system and discharges to the Dominguez Channel.

The easterly adjacent property, Subarea X3 currently sheet flows over land in the northwesterly direction to an existing storm drain inlet. There is an existing, non-operational stormwater sump pump located on the adjacent property that is intended to convey stormwater runoff to the Dominguez Channel. It is anticipated that due to the existing sump pump operational status, that stormwater eventually ponds and overflows onto the project site, near the northeast corner.

Refer to the Existing Conditions Hydrology Map located within Appendix A of this report for additional information.

4.0 PROPOSED CONDITIONS:

The proposed project consists 120 townhome units and a recreational area over approximately 5.63 acres. The proposed development include drive aisles, parking, landscaping, walkways and common open space areas. The site will be graded to collect runoff at various low points throughout the site in order to control the amount of imported fill during grading and the overall height of existing perimeter retaining walls. Stormwater runoff generated by the entire site will be directed towards the southwest and northwest corners of the site. The proposed development will utilize catch basins and an onsite area drain system to collect and convey to a proposed underground storm drain system. The stormwater runoff will be conveyed offsite via a proposed LACFCD connection to the existing 48" RCP storm drain system within West 178TH Street and ultimately discharging the Dominguez

Channel. Redirecting historic drainage patterns is not anticipated to cause any downstream capacity concerns since the stormwater runoff for both proposed and existing conditions ultimately discharge to the Dominguez Channel.

Each catch basin inlet will be equipped with Dvert System that will divert low flows to proposed Modular Wetlands System (MWS) Biofiltration Vaults for water quality treatment. Refer to the separate prepared Preliminary Low Impact Development (LID) Plan for reference.

It is anticipated that the City will reach out to the adjacent property owner, Subarea X3 and request the existing, non-operational stormwater sump pump be maintained/ repaired to prevent cross lot drainage. However, to support the proposed development and prevent potential run-on, a 24" storm drain pipe and catch basin is proposed along the northerly property line to provide conveyance of offsite storm water run-on from the easterly property, Subarea X3. Offsite stormwater runoff from the adjacent property will not be treated, only conveyed through the site.

For preliminary hydrologic purposes, initial subareas were determined based on the proposed preliminary grading and drainage design. Recommended impervious area ratio values from Los Angeles County Hydrology Manual 2006 – Appendix D were used in this study. During final engineering, impervious area will be calculated in more details to reflect more accurate peak flow value based on individual subareas.

Refer to the Proposed Conditions Hydrology Map located within Appendix A of the report.

5.0 METHODOLOGY:

The site was analyzed using the Los Angeles Hydrology Manual 2006 and HydroCalc Calculator Software. The initial subareas were analyzed for acreage, land-use, soil type, peak flow rate and time of concentration according to the Rational Method. The site was graded to allow for low points throughout the site to direct stormwater runoff to several areas equipped with curb inlet catch basins.

In this preliminary hydrology study, the recommended values per Los Angeles County Manual 1986 (Appendix D) were used for the percentage of impervious area for the existing and the proposed condition. In accordance with the Los Angeles County Hydrology Manual all habitable structures must have a finished floor elevation to allow 1 ft of freeboard during the 100-year storm event and the drop inlet catch basin and onsite conveyance storm drain pipes will be sized to convey runoff from the 25-year storm event. Catch basin & pipe sizing and 100-year water surface elevation calculations will be provided during final engineering.

Based on the allowable Q discharge rate provided LACFCD, onsite detention and a restriction orifice may be required in order to mitigate the increased post-developed peak flow rate and volume.

6.0 RESULTS:

Hydrology Summary

Offsite Tributary Flow:

Site 2-year peak storm flow = 2.82 cubic feet per second (cfs) Site 25-year peak storm flow = 9.06 cfs Site 50-year peak storm flow = 10.86 cfs Site 100-year peak storm flow = 12.81 cfs Tc = 9.00 min

Onsite Existing Conditions

Existing Conditions tributary to Northwest Corner: Site 2-year peak storm flow = 2.92 cfs Site 25-year peak storm flow = 8.09 cfs Site 50-year peak storm flow = 9.22 cfs Site 100-year peak storm flow = 10.34 cfs Time of Concertation (Tc) = 5.00 minute (min)

Existing Conditions tributary to Southwest Corner:

Site 2-year peak storm flow = 3.24 cfsSite 25-year peak storm flow = 7.57 cfsSite 50-year peak storm flow = 8.62 cfsSite 100-year peak storm flow = 9.67 cfsTc = 5.00 min

Existing Conditions (Total Onsite):

Total site 2-year peak storm flow = 6.16 cfs Total site 25-year peak storm flow = 15.66 cfs Total site 50-year peak storm flow = 17.84 cfs Total site 100-year peak storm flow = 20.01 cfs

Onsite Proposed Conditions

<u>Proposed Condition tributary to A1:</u> Site 2-year peak storm flow = 0.43 cfsSite 25-year peak storm flow = 1.22 cfsSite 50-year peak storm flow = 1.39 cfsSite 100-year peak storm flow = 1.56 cfsTc = 5.00 min

<u>Proposed Condition tributary to A2:</u> Site 2-year peak storm flow = 0.79 cfs Site 25-year peak storm flow = 2.39 cfs Site 50-year peak storm flow = 2.73 cfs Site 100-year peak storm flow = 3.06 cfs Tc = 5.00 min <u>Proposed Condition tributary to A3:</u> Site 2-year peak storm flow = 1.03 cfsSite 25-year peak storm flow = 3.31 cfsSite 50-year peak storm flow = 3.77 cfsSite 100-year peak storm flow = 4.23 cfsTc = 5.00 min

<u>Proposed Condition tributary to A4:</u> Site 2-year peak storm flow = 0.50 cfsSite 25-year peak storm flow = 1.42 cfsSite 50-year peak storm flow = 1.62 cfsSite 100-year peak storm flow = 1.81 cfsTc = 5.00 min

<u>Proposed Condition tributary to A5:</u> Site 2-year peak storm flow = 0.80 cfsSite 25-year peak storm flow = 2.71 cfsSite 50-year peak storm flow = 3.08 cfsSite 100-year peak storm flow = 3.77 cfsTc = 5.00 min

<u>Proposed Condition tributary to A6:</u> Site 2-year peak storm flow = 1.09 cfsSite 25-year peak storm flow = 3.73 cfsSite 50-year peak storm flow = 4.25 cfsSite 100-year peak storm flow = 5.12 cfsTc = 6.00 min

<u>Proposed Conditions (Total Onsite)</u>: Total site 2-year peak storm flow = **4.64 cfs** Total site 25-year peak storm flow = **14.78 cfs** Total site 50-year peak storm flow = **16.84 cfs** Total site 100-year peak storm flow = **19.55 cfs**

Percent Difference:

 Δ Total site 2-year peak storm flow = 4.64/6.16 = -29.9% Δ Total site 25-year peak storm flow = 14.78/15.66 = -5.6% Δ Total site 50-year peak storm flow = 16.84/17.84 = -5.6% Δ Total site 100-year peak storm flow = 19.55/20.01 = -2.3%

Note: All time of concentrations indicated above refer to 100-year storm event.

Catch Basin Sizing

Catch basin Sizing will be analyzed for the 25-year storm event peak flow rates and will be provided during final engineering.

Pipe Sizing

Pipe Sizing will be analyzed using WSPG software for the 25-year storm event peak flow rates and will be provided during final engineering.

100-Year Water Surface Elevations

Water surface elevations for the 100-year storm event peak flow rates will verify that the proposed finish floor elevations are set at least 1' above the water surface elevation and will be calculated during final engineering.

7.0 CONCLUSION:

The results from this preliminary hydrology study utilizing HydroCalc software provided by Los Angeles County Department of Public Works demonstrate that the proposed stormwater peak flow from the subject site will be generally lower than the existing condition peak flow as indicated in the hydrology summary results in Section 6 of this report. Proposed peak flow being lower has mainly to do with the fact that the existing site is being proposed for a zone change from commercial to residential land use. For this reason, this would lower the impervious area of the land which causes lower runoff flow rate and higher time of concentration. During final engineering, the impervious area calculation will be finetuned for both proposed and existing condition to reflect more accurate peak flow values based on individual subareas.

Since the stormwater runoff generated by the entire project site is being proposed to be conveyed to the existing LACFCD 48" RCP BI 0432 – Line C ultimately discharging into the Dominguez Channel, the proposed peak flow rate compared to the allowable Q discharge rate will need to be verified with LACFCD for compliance.

During a heavy rainfall, the site was graded to allow for multiple low points equipped with curb inlet catch basins throughout the entire site to accommodate smaller drainage area to mitigate stormwater ponding in one spot. In an event of overflowing, the proposed grading will facilitate the overflow by draining half of the project site to northwest corner, matching the historic drainage condition, and providing wall knockouts for emergency overflow. The proposed catch basins will also be equipped with Dvert System to divert low flows to proposed Modular Wetlands System (MWS) Biofiltration vaults for water quality treatment which will be provided as a separate LID plan and internal bypass systems to convey larger storm event overflow conditions. Detention is anticipated as not required since the peak flow runoff from the proposed preliminary condition is lower than that of the existing condition. However, the peak flow runoff will need to be re-evaluated based on the LACFCD Allowable Q Discharge Rate.

8.0 DESIGN ASSUMPTIONS:

- 1. The property is located in the City of Gardena, Los Angeles County rainfall region.
- 2. 100-year storm event flood level protection analysis required for habitable structures per the requirements of the Los Angeles Hydrology Manual.
- 3. 25-year storm event flood level protection analysis required for storm drain system per the requirements of the Los Angeles County Hydrology Manual.
- 4. Detention maybe required for the storm drain system to mitigate existing and proposed conditions peak flow rate and time of concertation per the requirements Los Angeles County Hydrology Manual based on the LACFCD Allowable Q Discharge Rate.
- 5. Site located within Soil Type "13" per the LACDPW Hydrology Map GIS Viewer. (See Appendix C of this report for reference)
- 6. 50-year storm event 24-hr rainfall depth = 5.9 per the LACDPW Hydrology Map GIS Viewer. (Refer to Appendix C of this report for reference)
- 7. Assumed Townhouses Land Use for proposed conditions and Commercial Storage Land Use for existing conditions. The values for impervious area were then selected from the recommended values in Los Angeles County Hydrology Manual 2006 Appendix D.
- 8. Peak flow rates and time of concentrations were calculated using the HydroCalc Software provided by the Los Angeles County Department of Public Works.

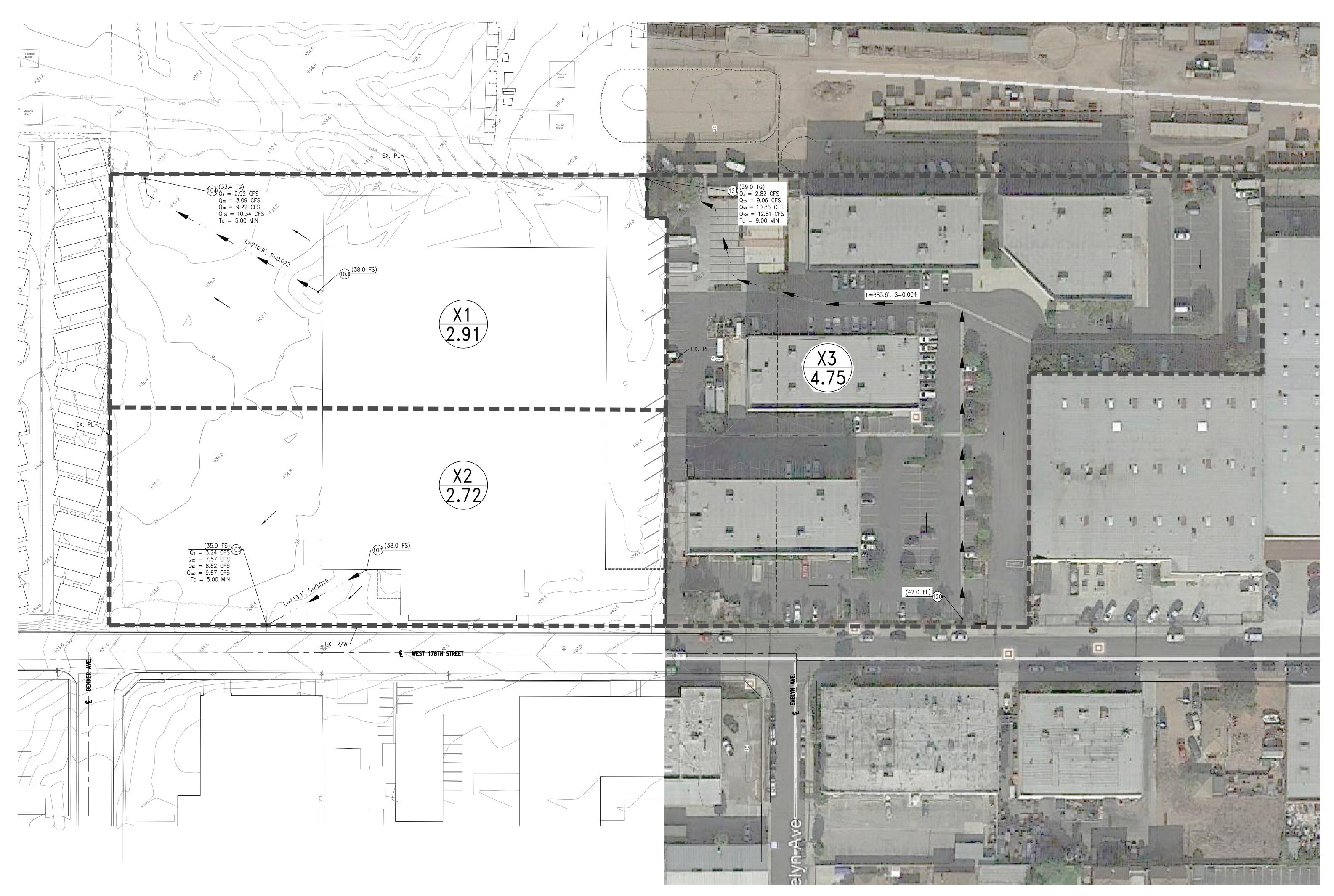
9.0 REFERENCES:

- 1. Los Angeles County Hydrology Manual 2006
- 2. HydroCalc Software provided by LACDPW
- 3. Tentative Tract Maps prepared by C&V Consulting, Inc. dated November 2018
- 4. LACDPW Hydrology Map GIS Viewer.
- 5. LACFCD 48" RCP BI 0432 Line C Storm Drain As-built Plan

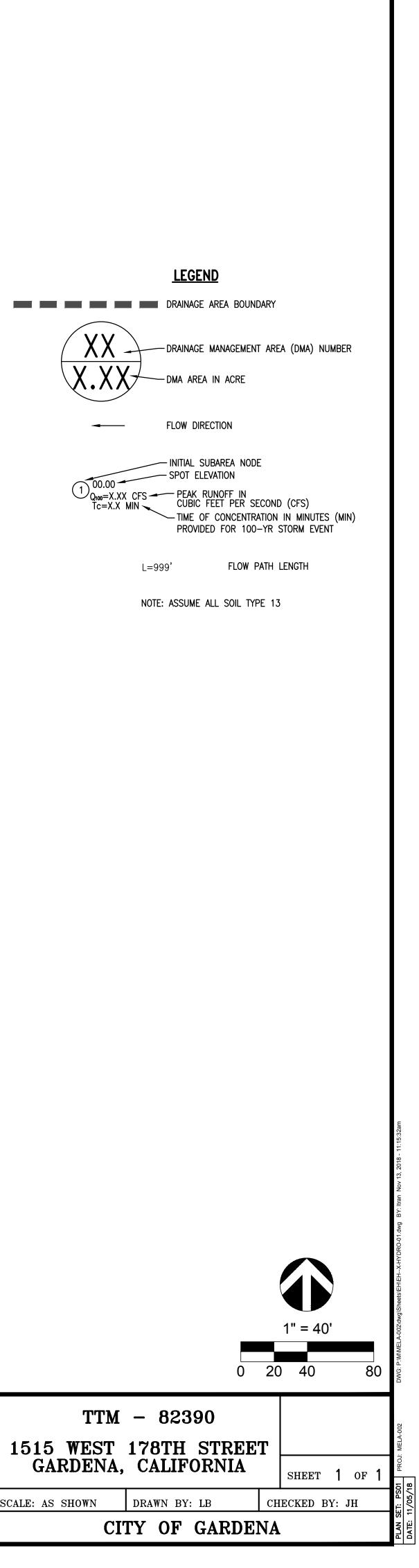
APPENDIX A HYDROLOGY MAPS

Existing Conditions Hydrology Map

EXISTING CONDITIONS PRELIMINARY HYDROLOGY MAP FOR TENTATIVE TRACT NO. 82390 CITY OF GARDENA COUNTY OF LOS ANGELES, STATE OF CALIFORNIA



			REVISIONS			OWNER/DEVELOPER
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IOMES

MELIA HOMES 8951 RESEARCH DR. #100 IRVINE, CA 92618 (949) 759–4367

PREPARED BY :

6 ORCHARD, SUITE 200 LAKE FOREST, CA 92630 CONSULTING, INC. F. 949.916.3800 CIVIL ENGINEERING LAND PLANNING & SURVEYING CVC-INC.NET

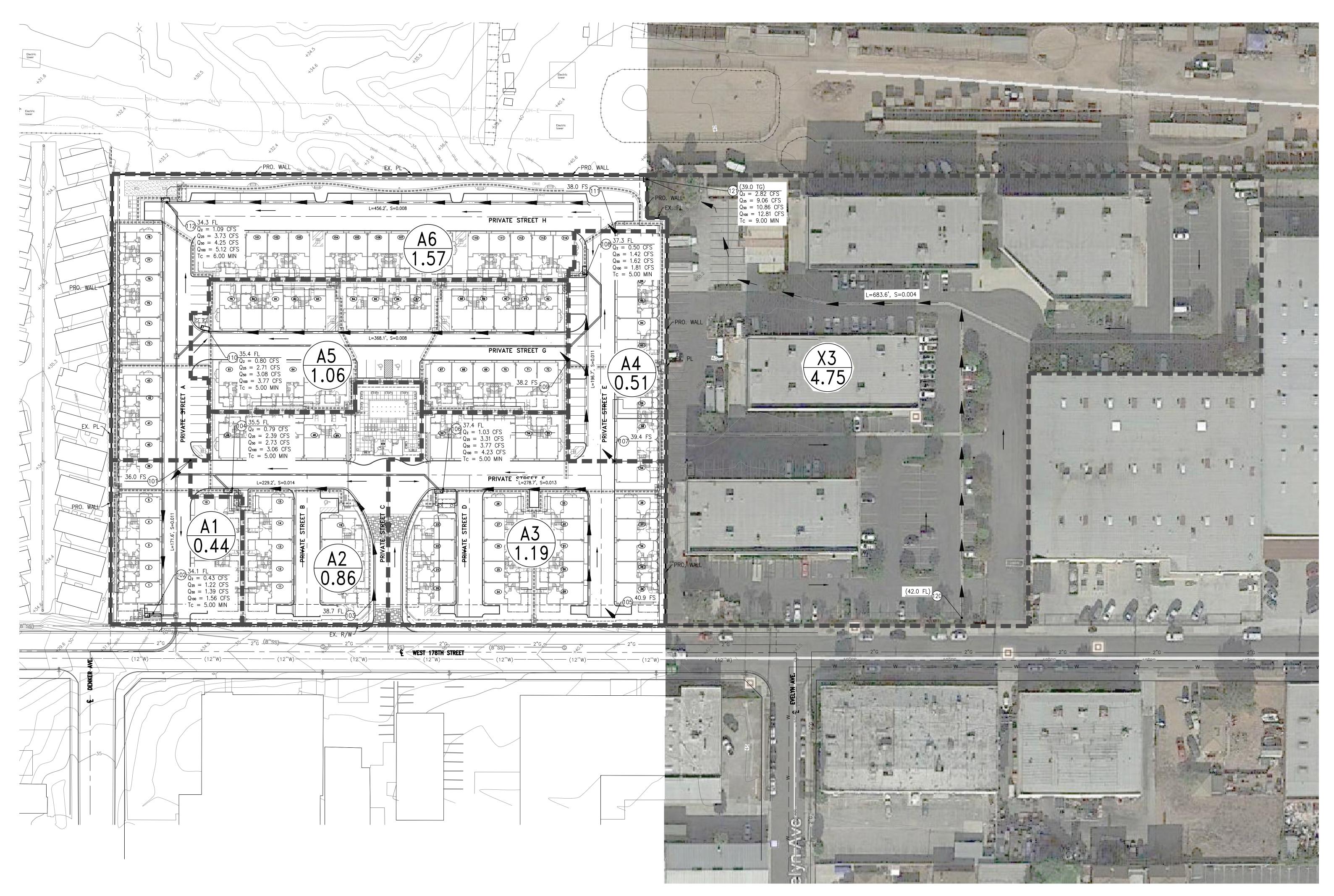
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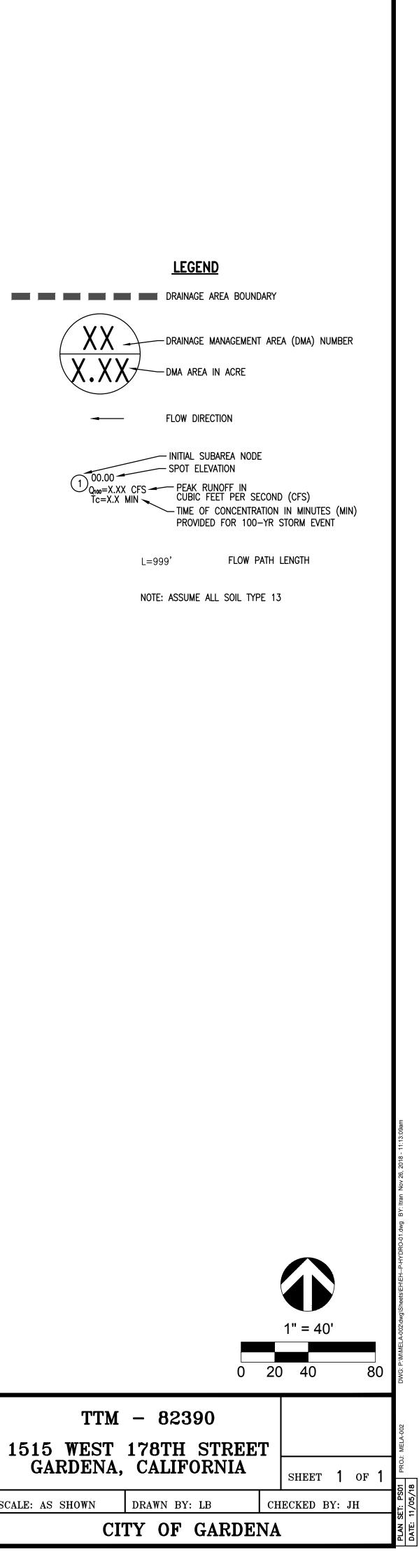
SCALE: AS SHOWN

Proposed Conditions Preliminary Hydrology Map

PROPOSED CONDITIONS PRELIMINARY HYDROLOGY MAP FOR TENTATIVE TRACT NO. 82390 CITY OF GARDENA COUNTY OF LOS ANGELES, STATE OF CALIFORNIA



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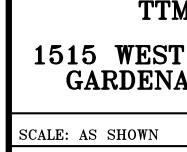
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MELIA HOMES 8951 RESEARCH DR. #100 IRVINE, CA 92618 (949) 759–4367

PREPARED BY :

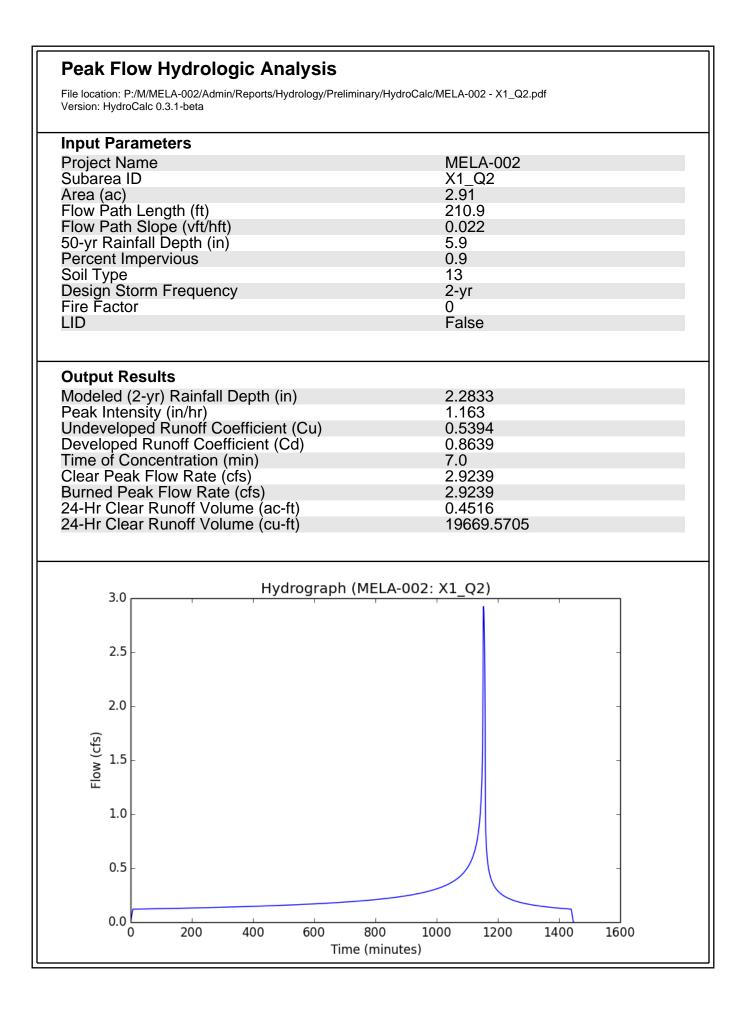
6 ORCHARD, SUITE 200 LAKE FOREST, CA 92630 T. 949.916.3800 CONSULTING, INC. F. 949.916.3805 CIVIL ENGINEERING LAND PLANNING & SURVEYING CVC-INC.NET

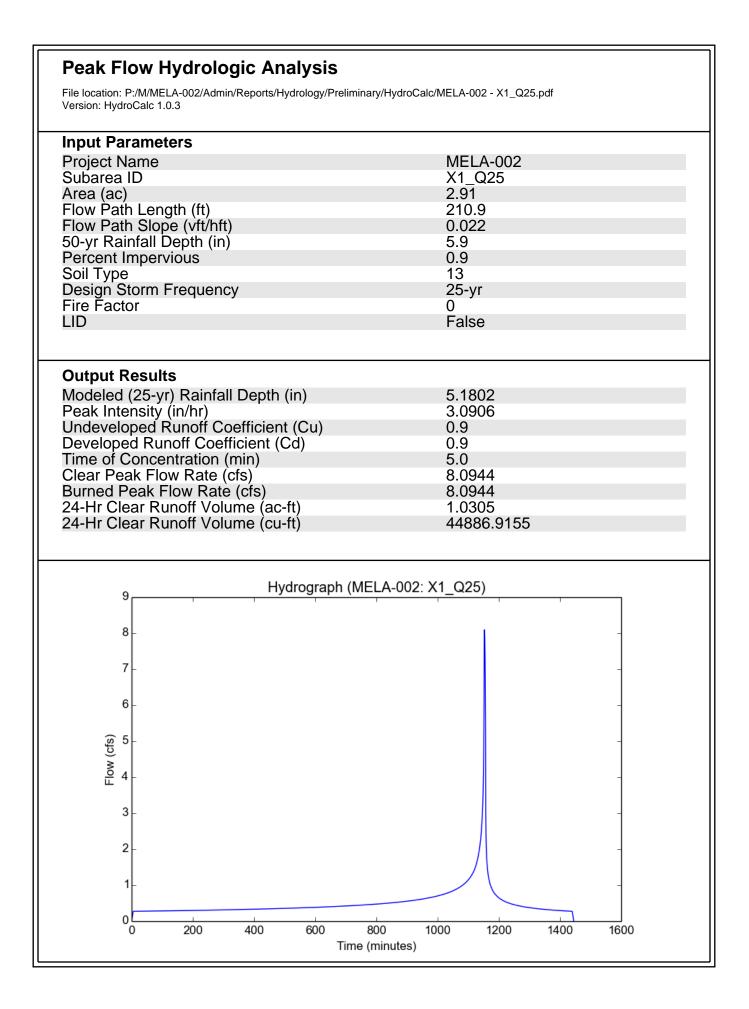


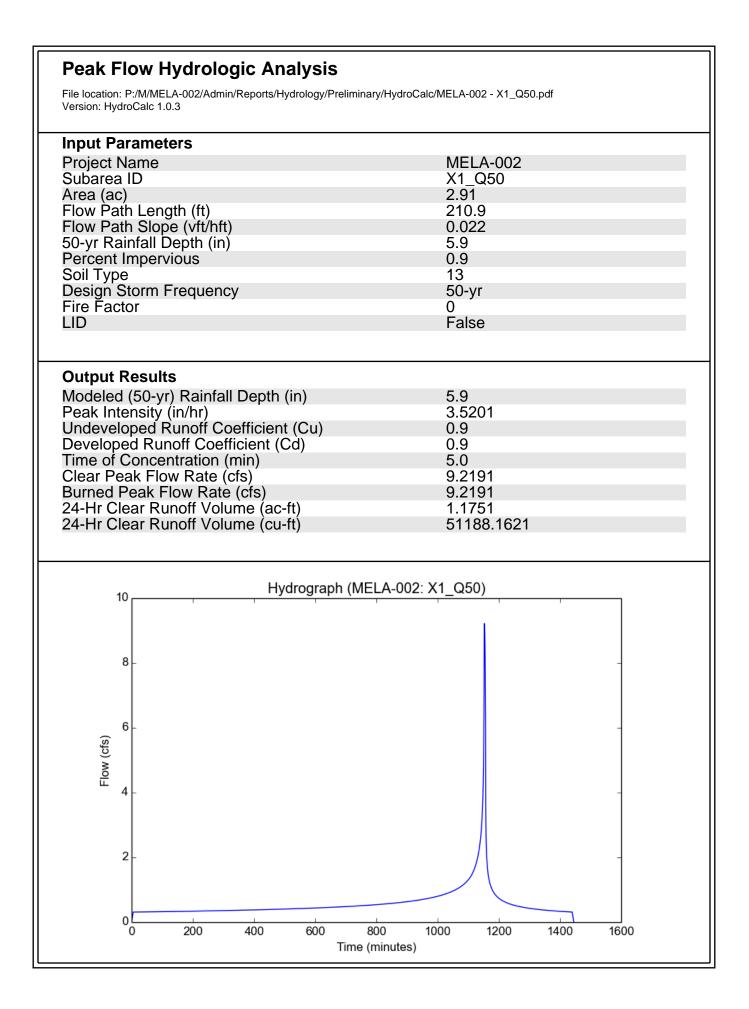


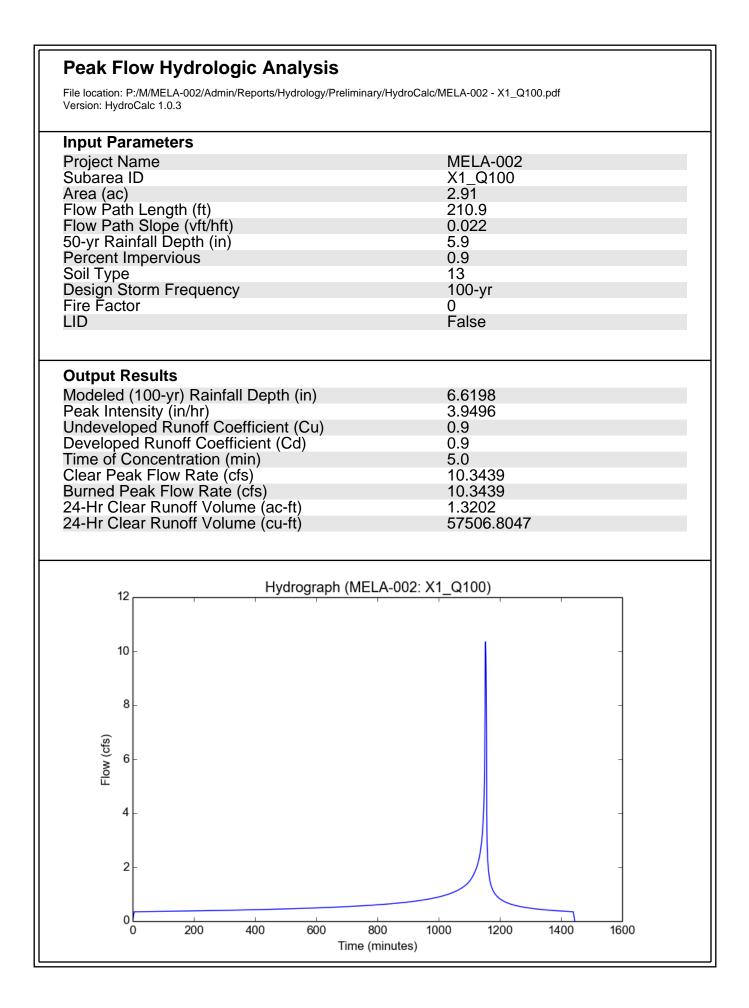
APPENDIX B HYDROLOGY CALCULATIONS

Area X1 - Existing Conditions Hydrology Calculations (2, 25, 50, 100-year Storm Event)

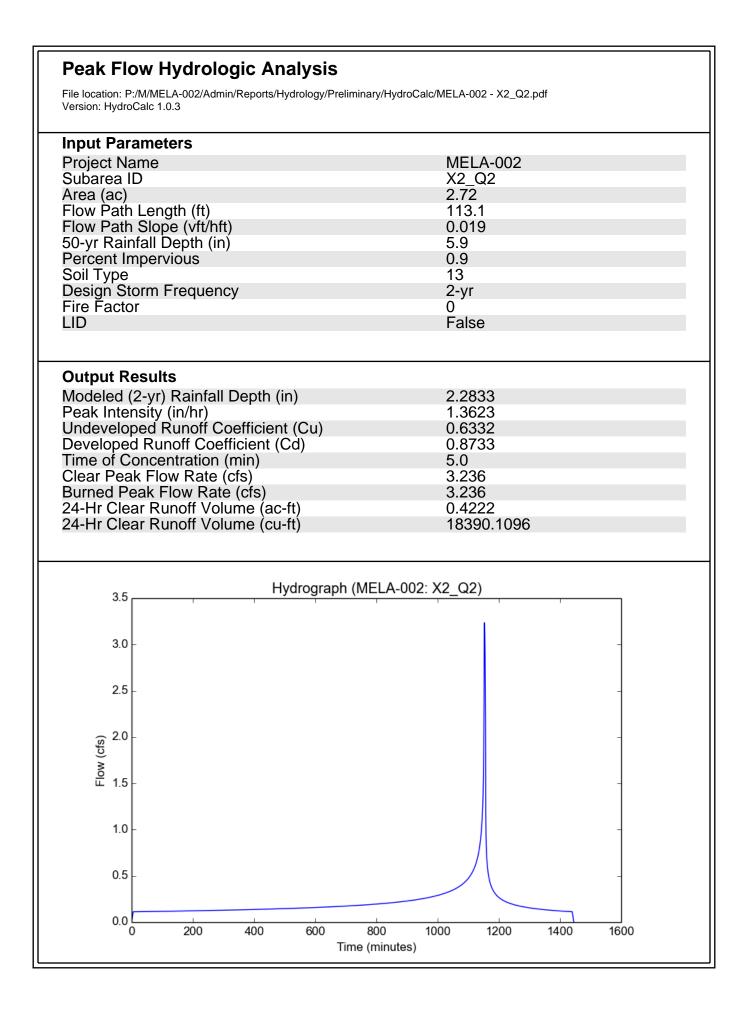


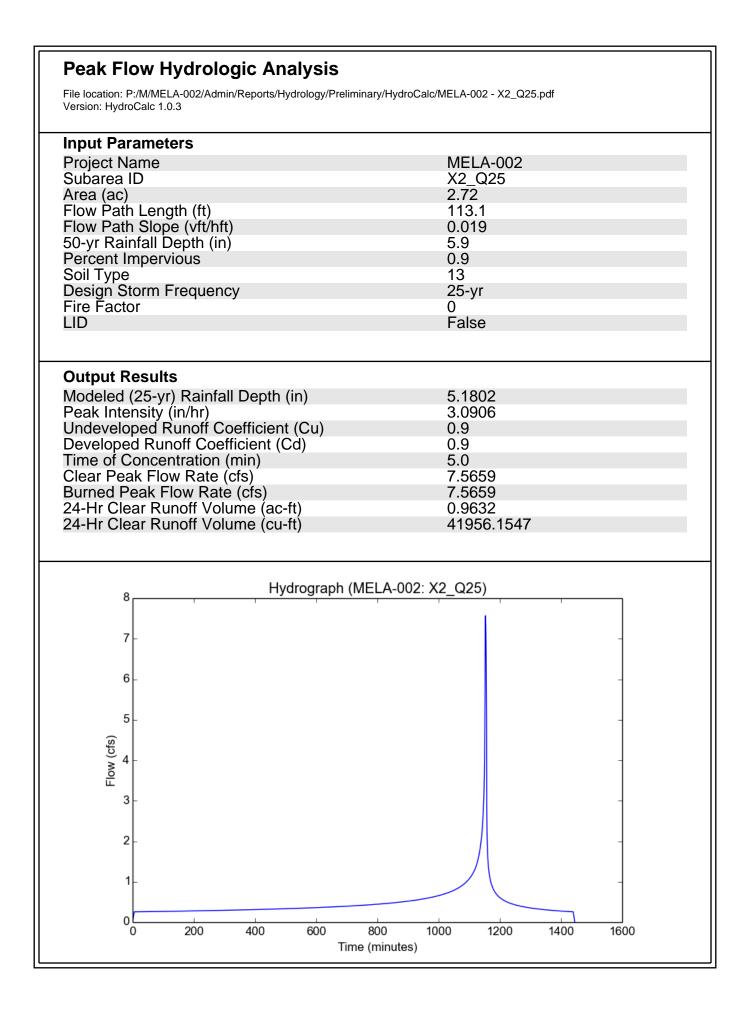


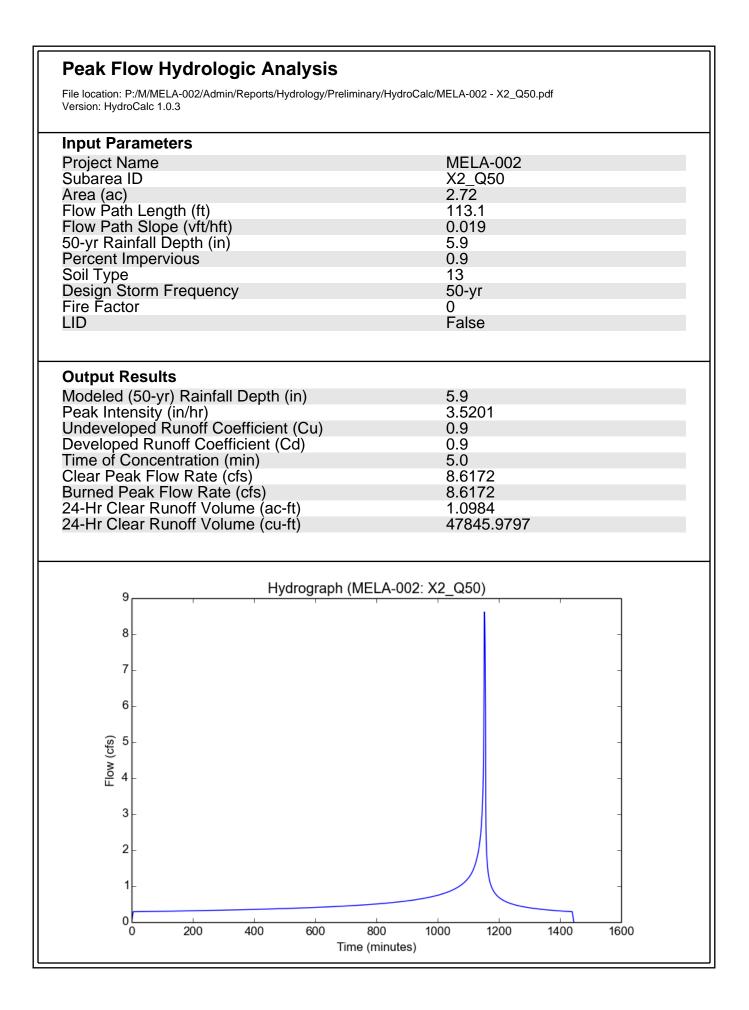


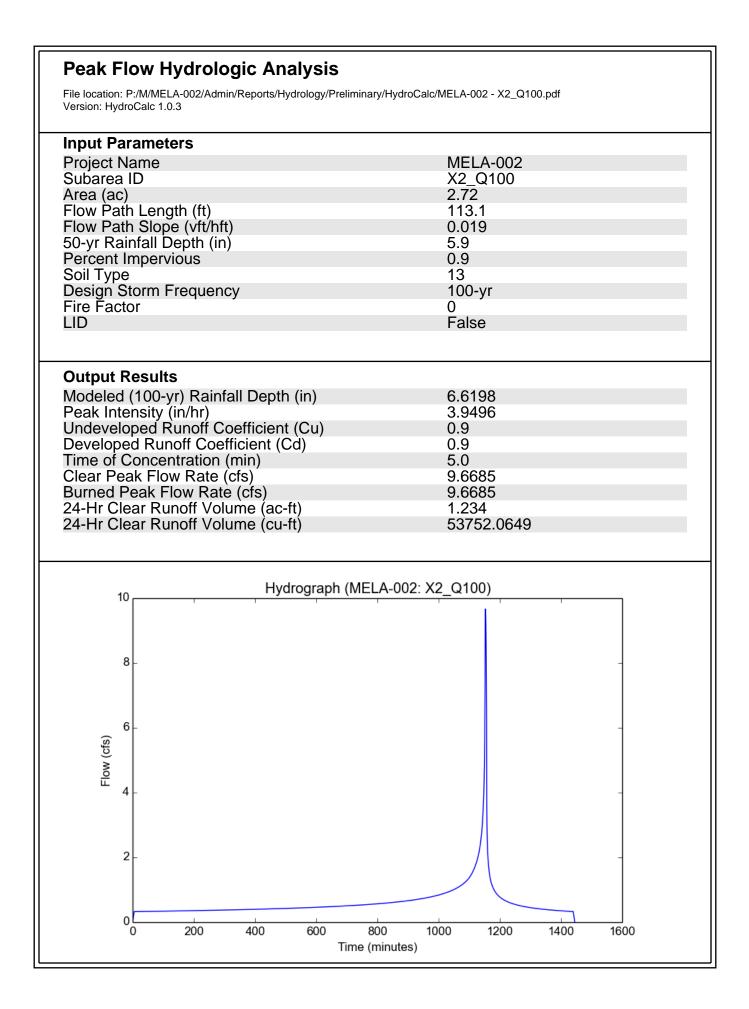


Area X2 - Existing Conditions Hydrology Calculations (2, 25, 50, 100-year Storm Event)

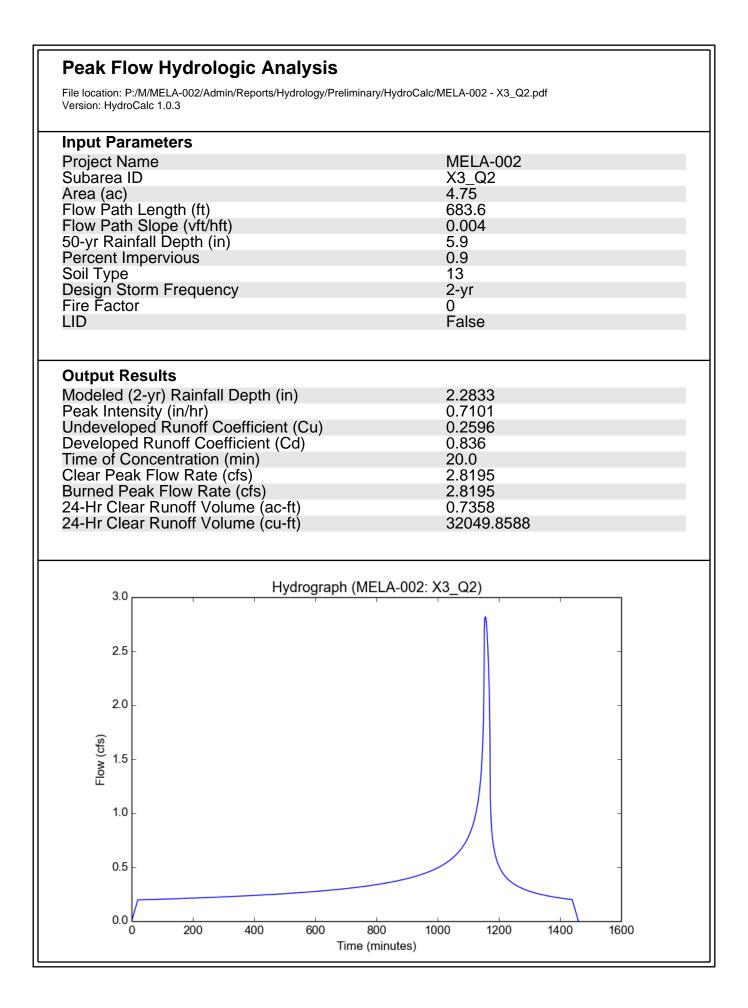


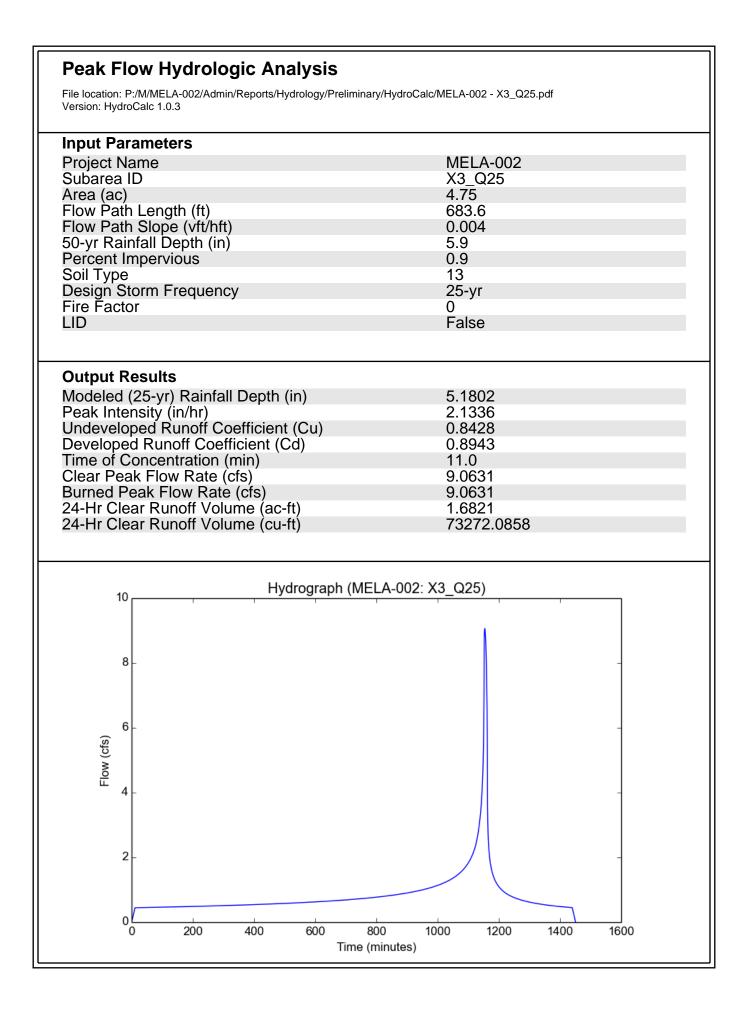


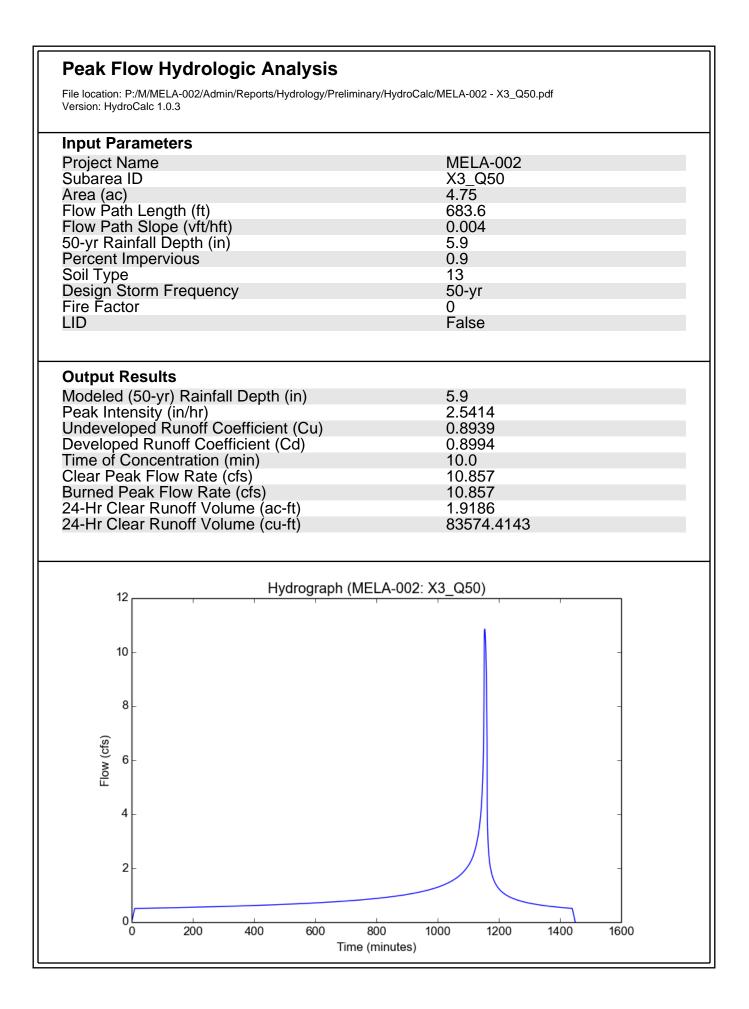


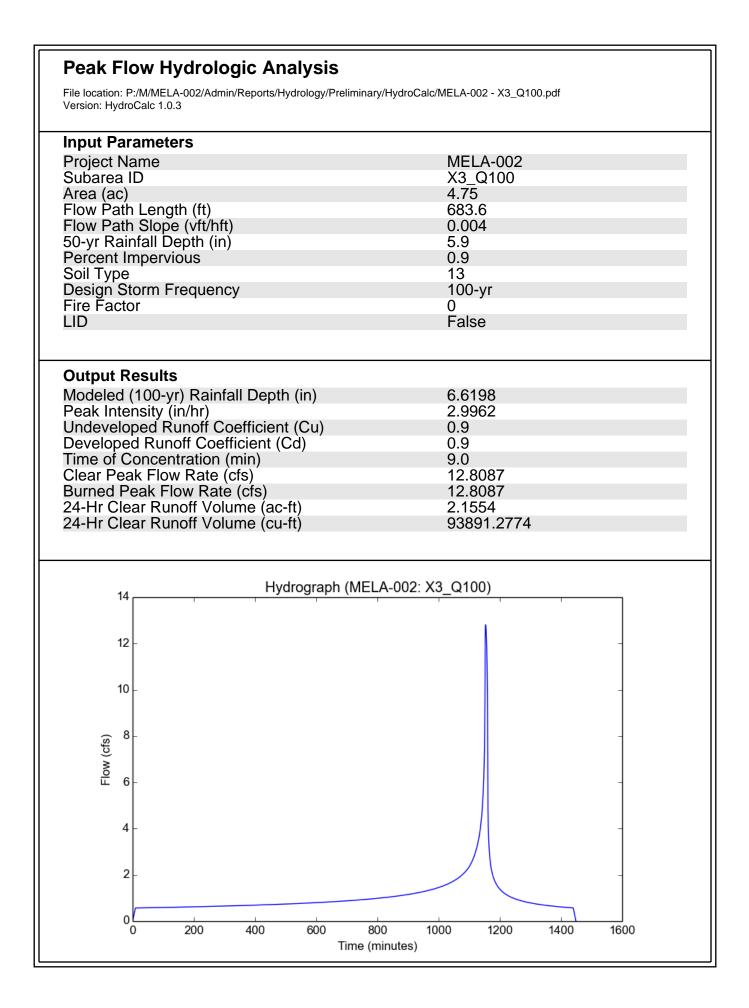


Area X3 - Existing Conditions Hydrology Calculations (2, 25, 50, 100-year Storm Event)

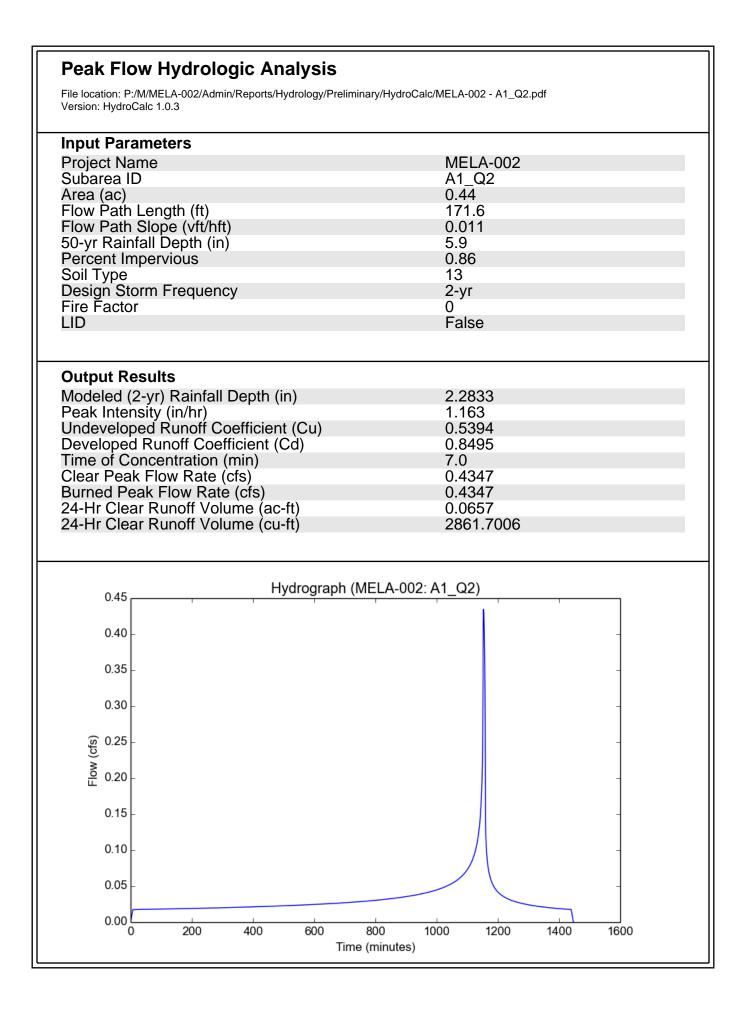


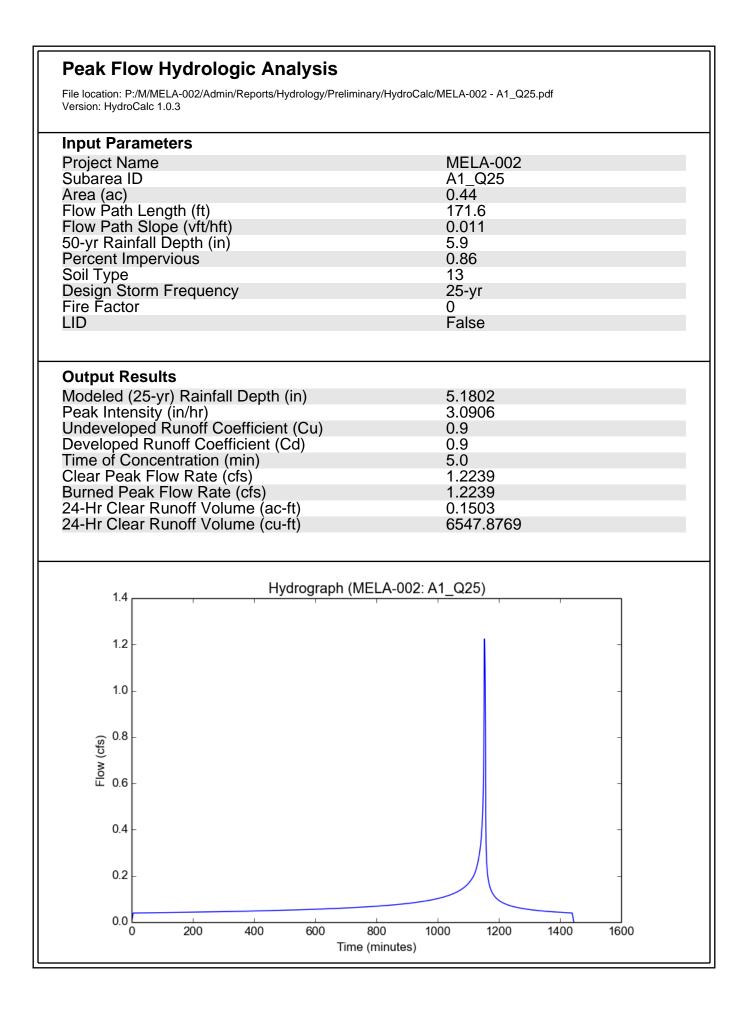


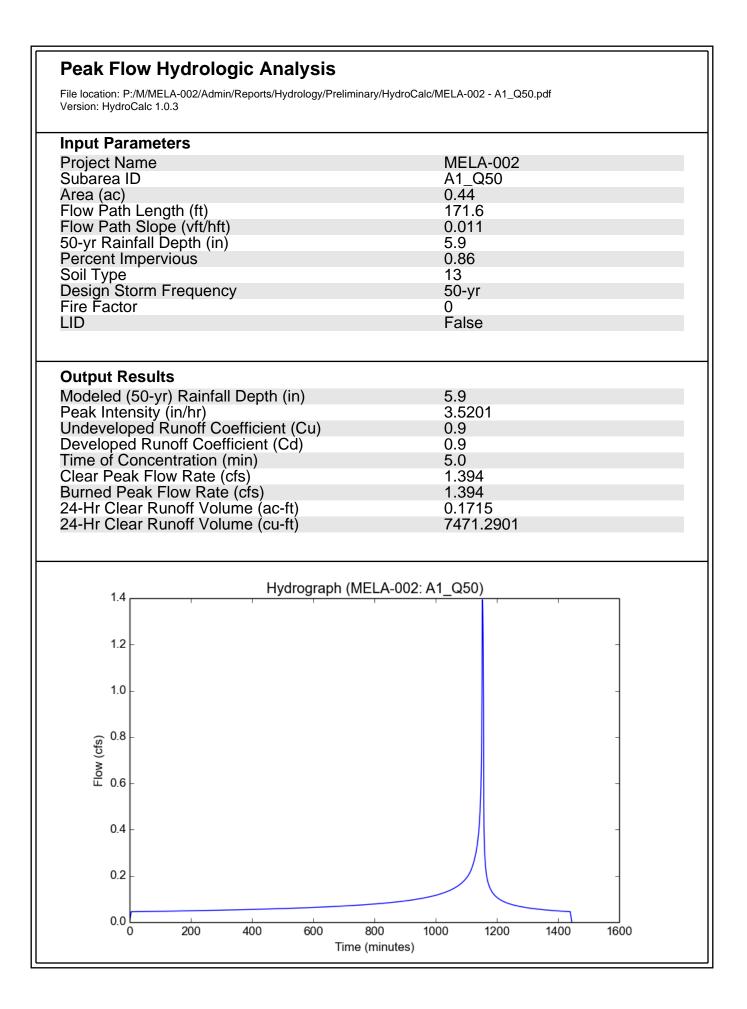


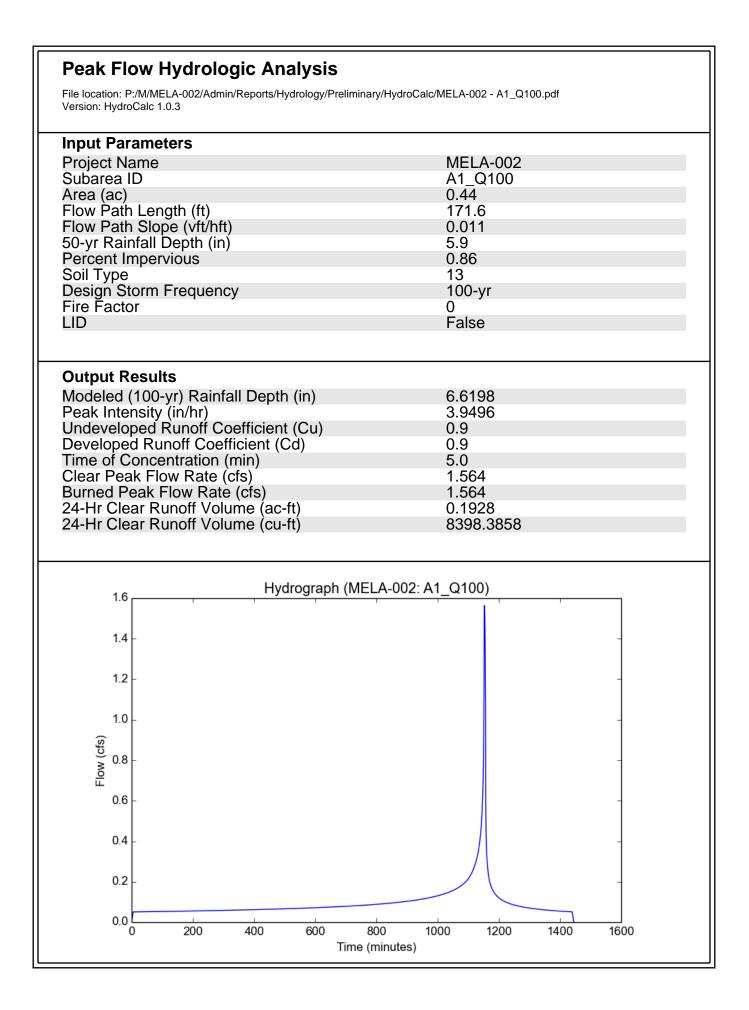


Area A1 - Proposed Conditions Hydrology Calculations (2, 25, 50, 100-year Storm Event)

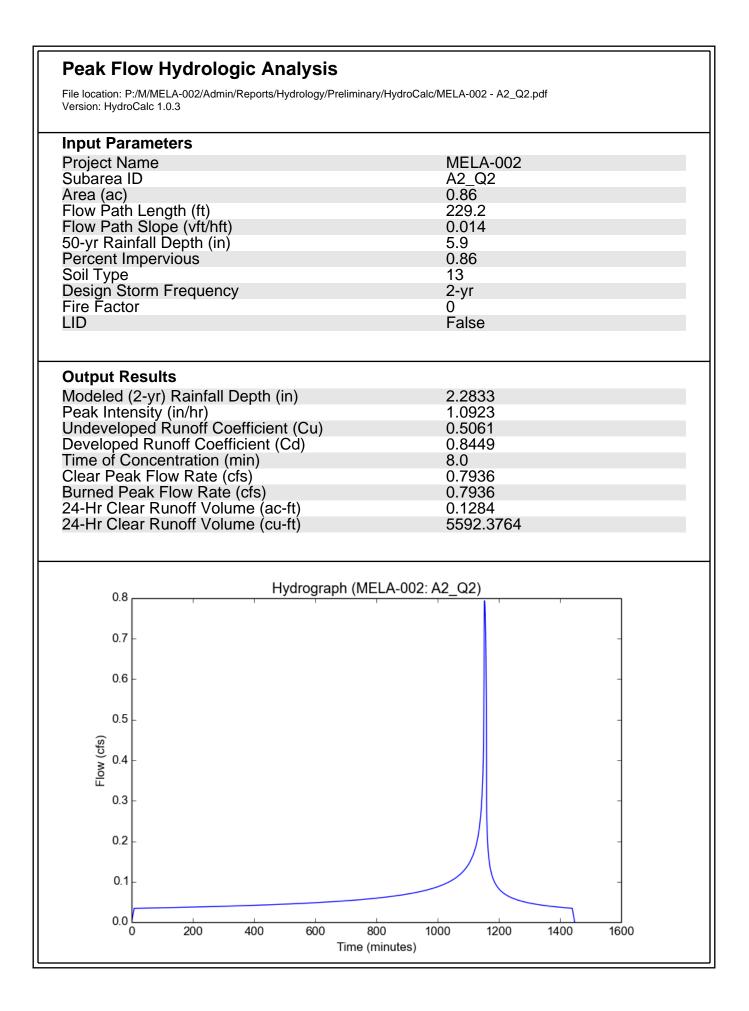


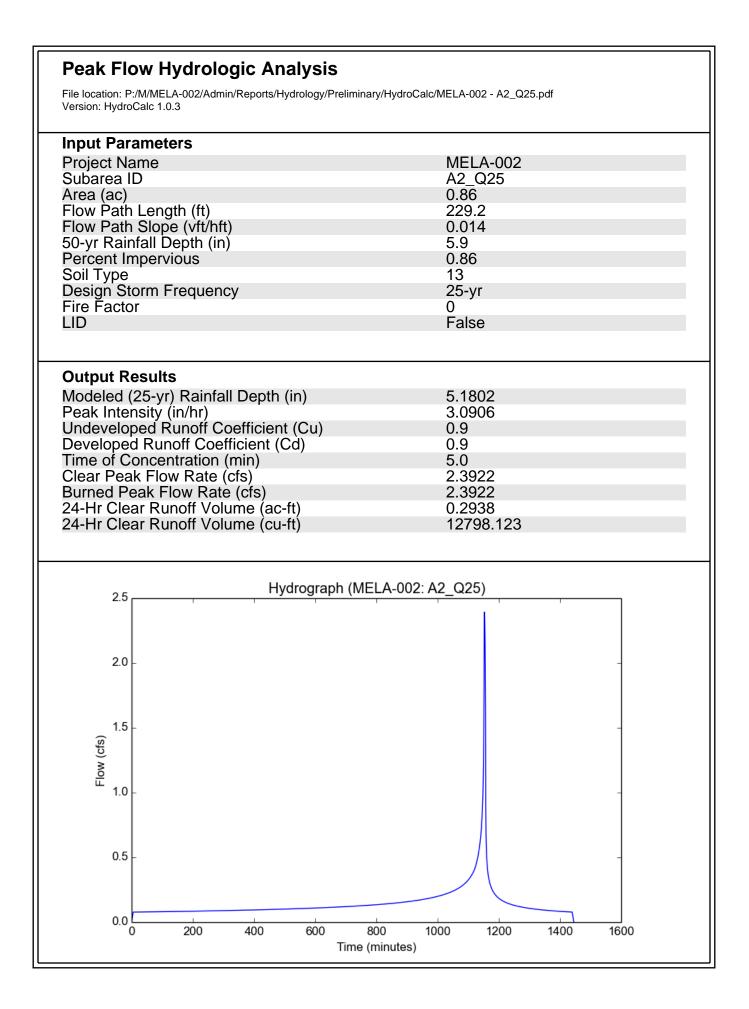


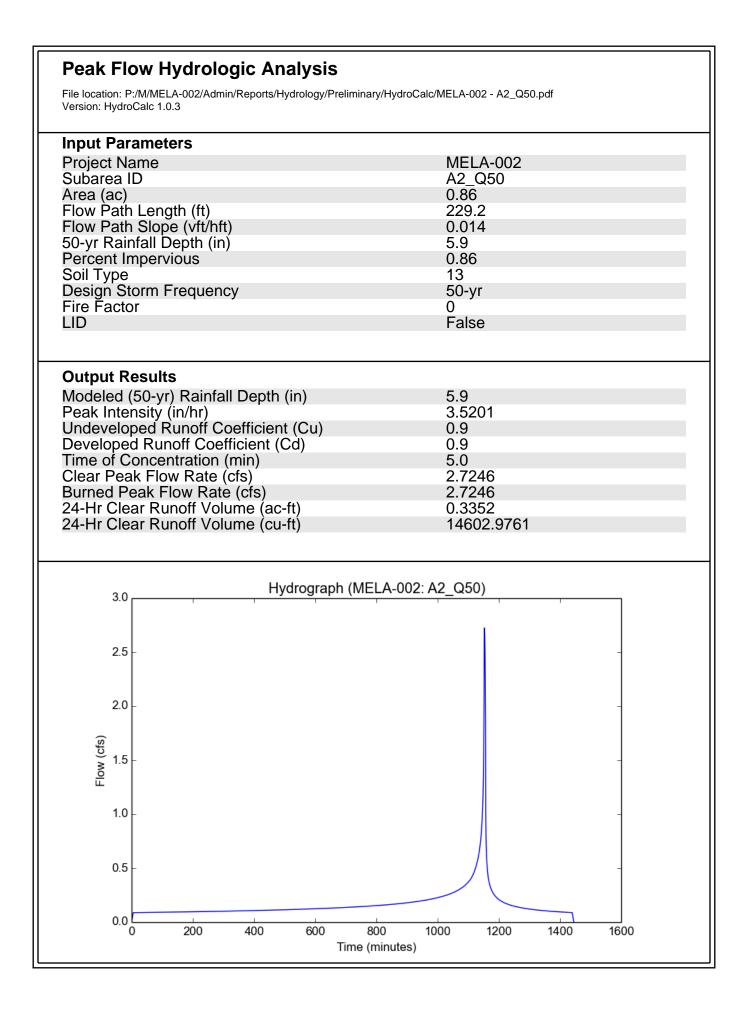


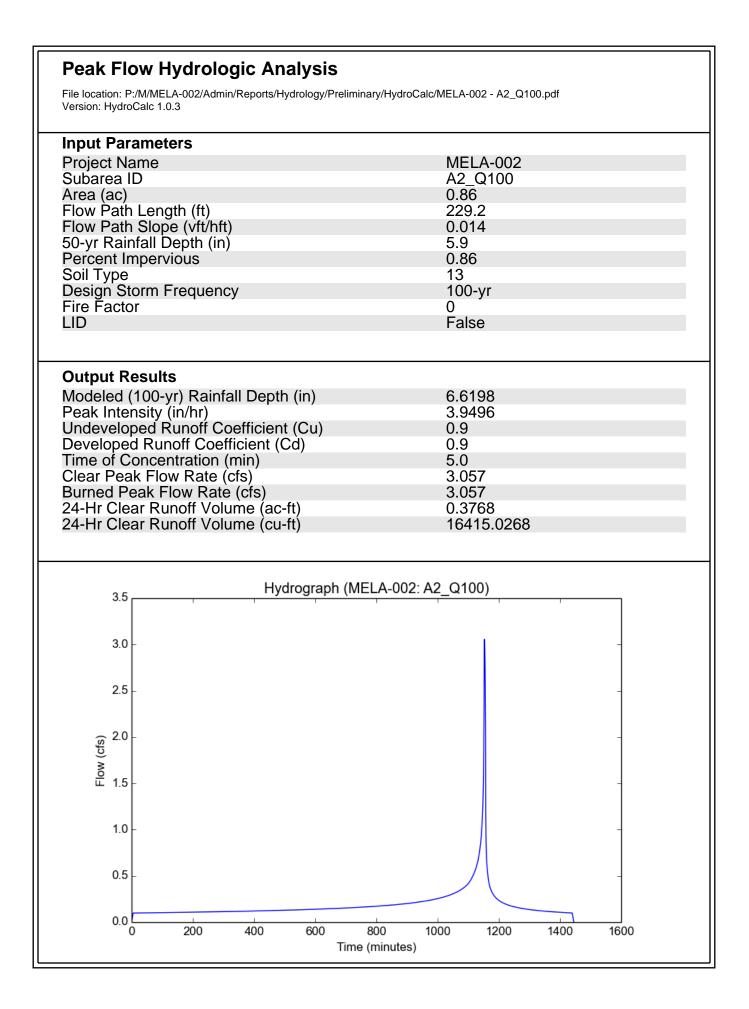


Area A2 - Proposed Conditions Hydrology Calculations (2, 25, 50, 100-year Storm Event)

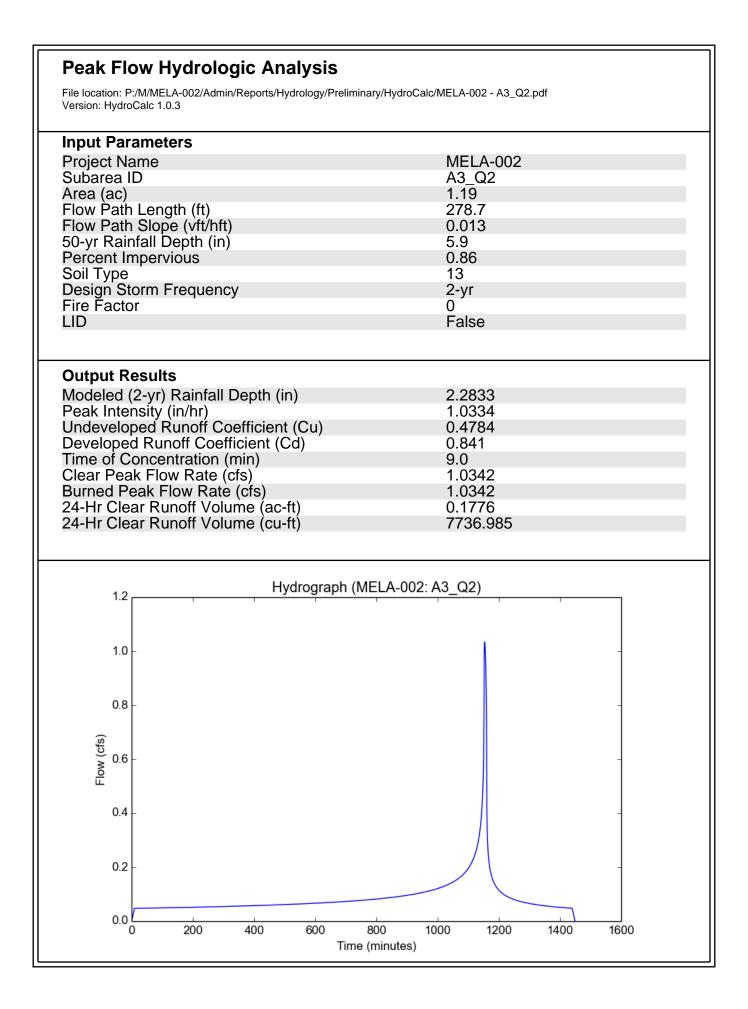


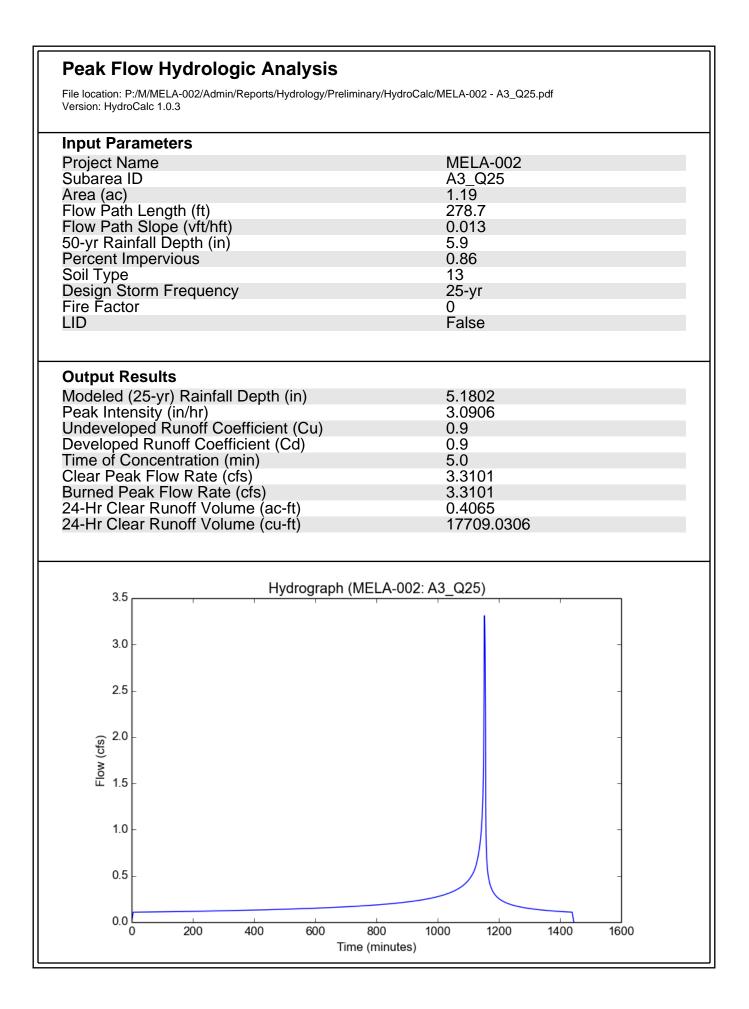


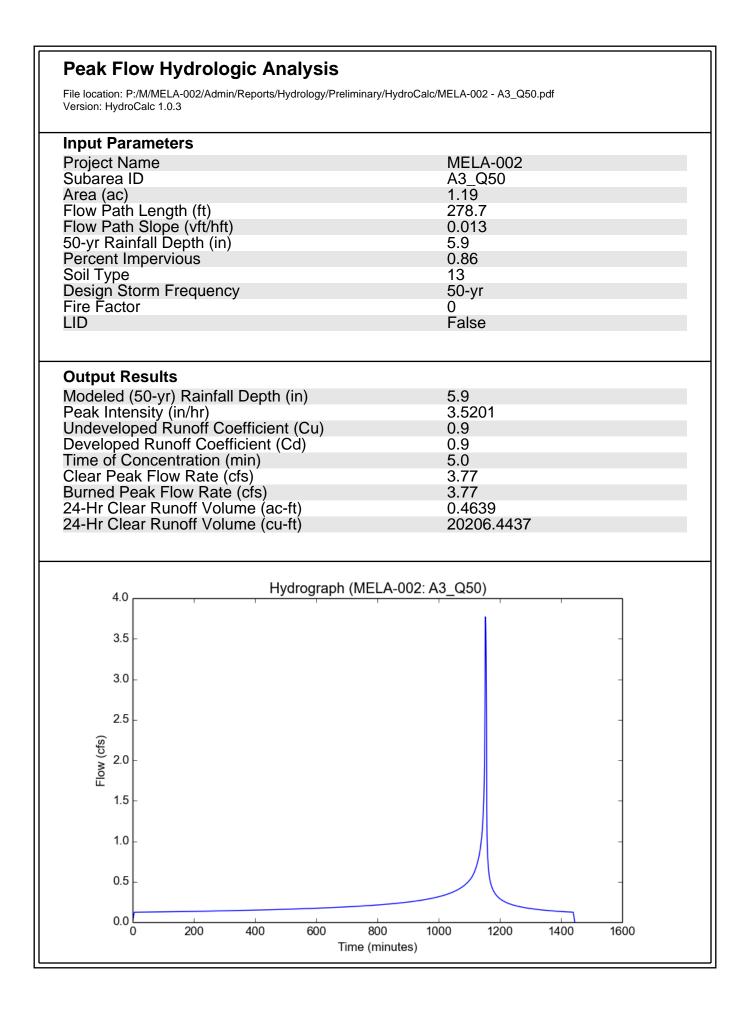


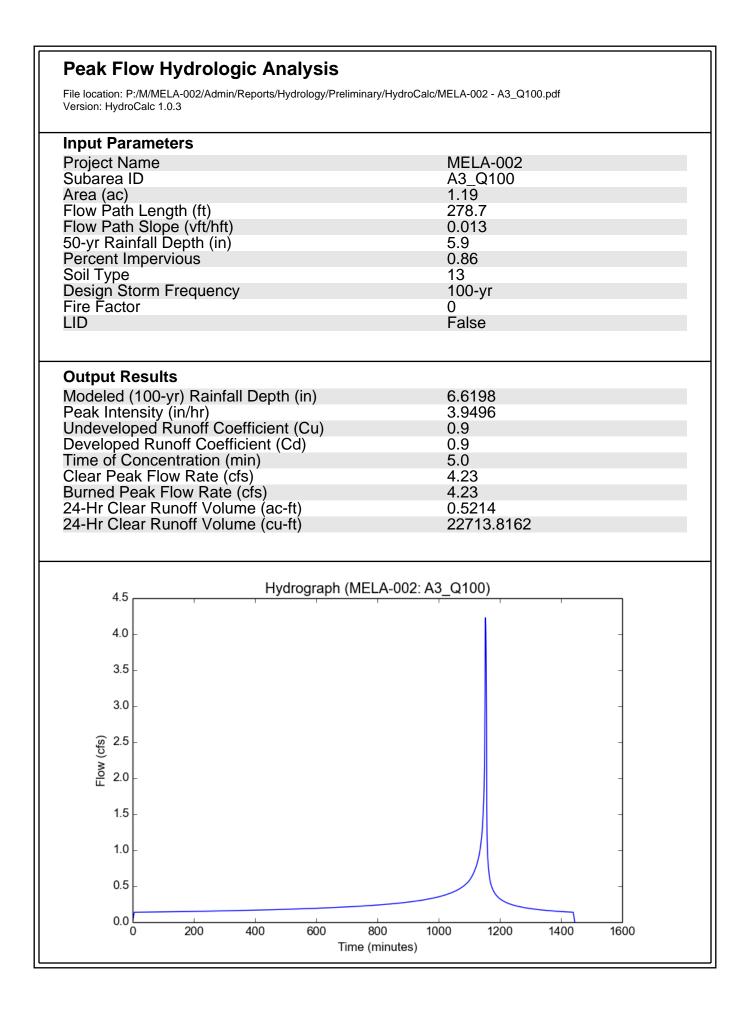


Area A3 - Proposed Conditions Hydrology Calculations (2, 25, 50, 100-year Storm Event)

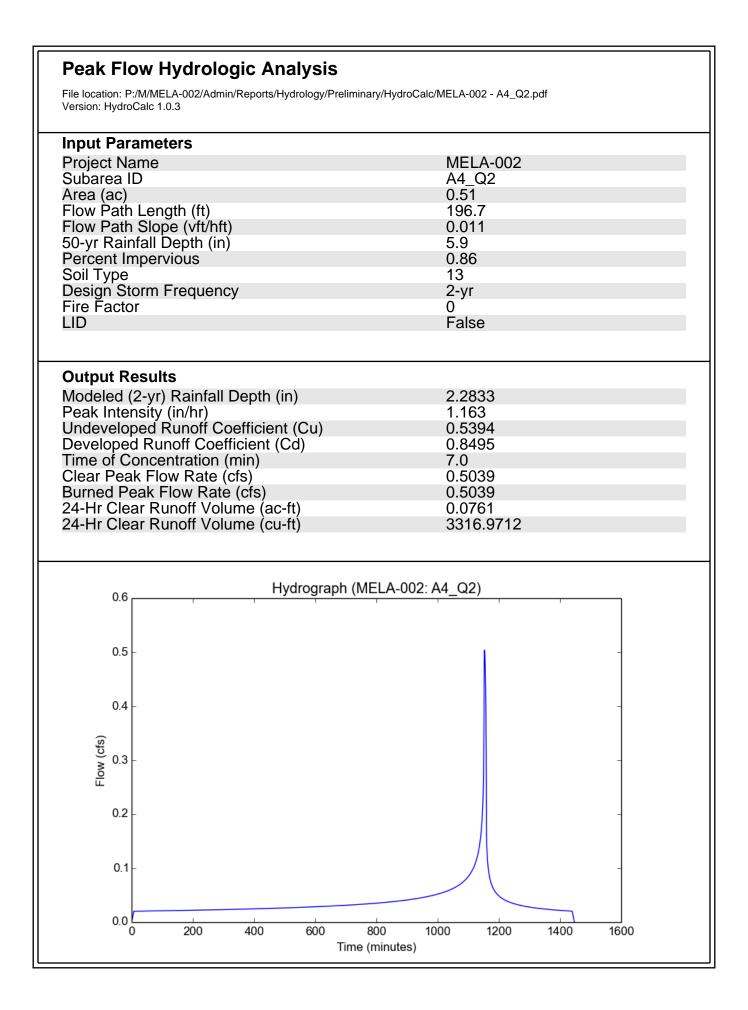


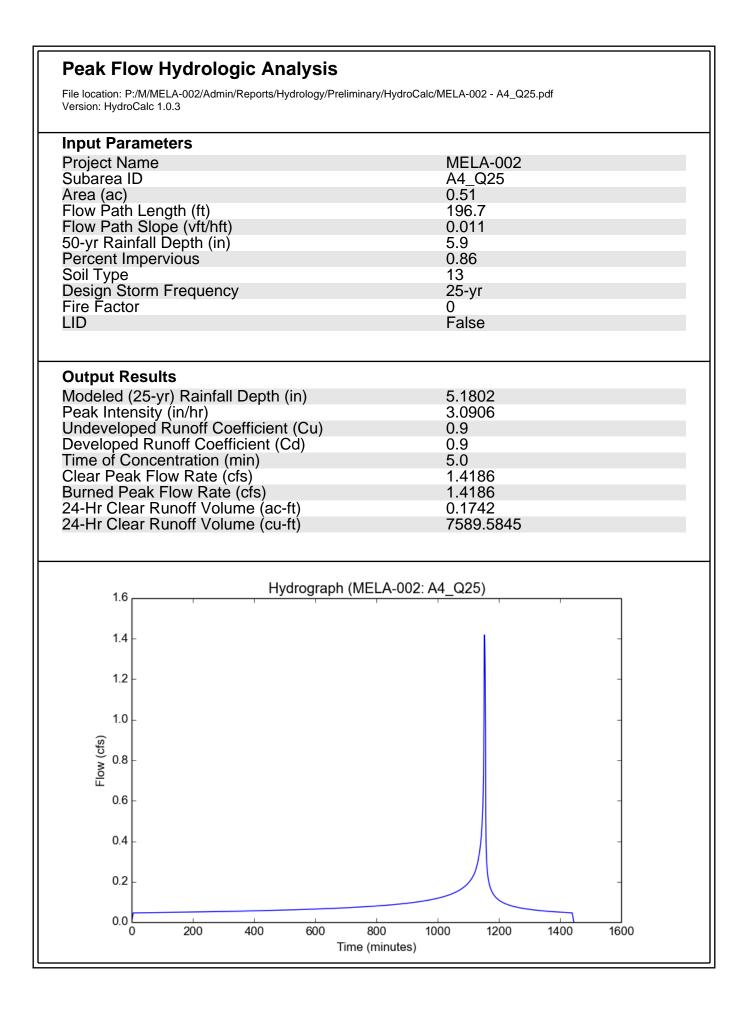


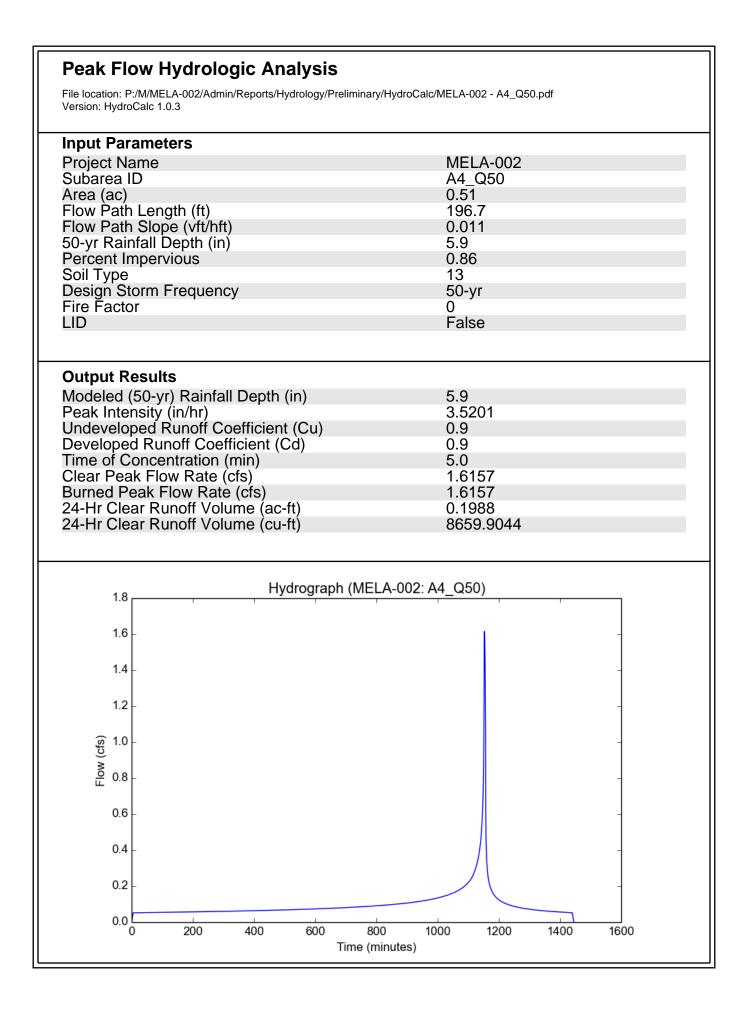


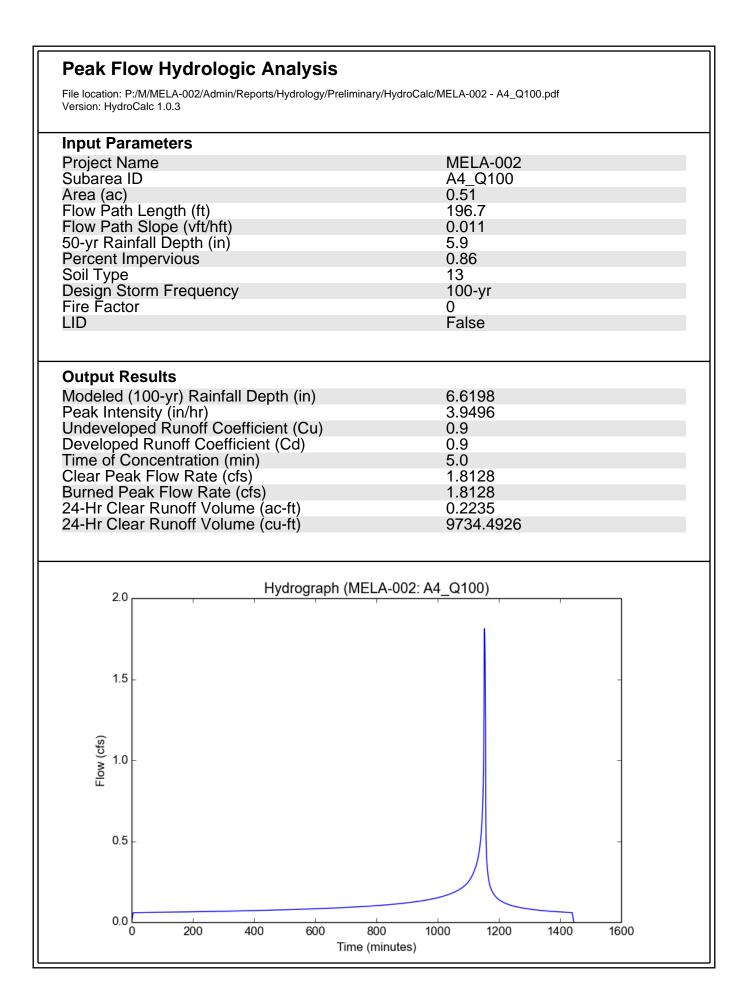


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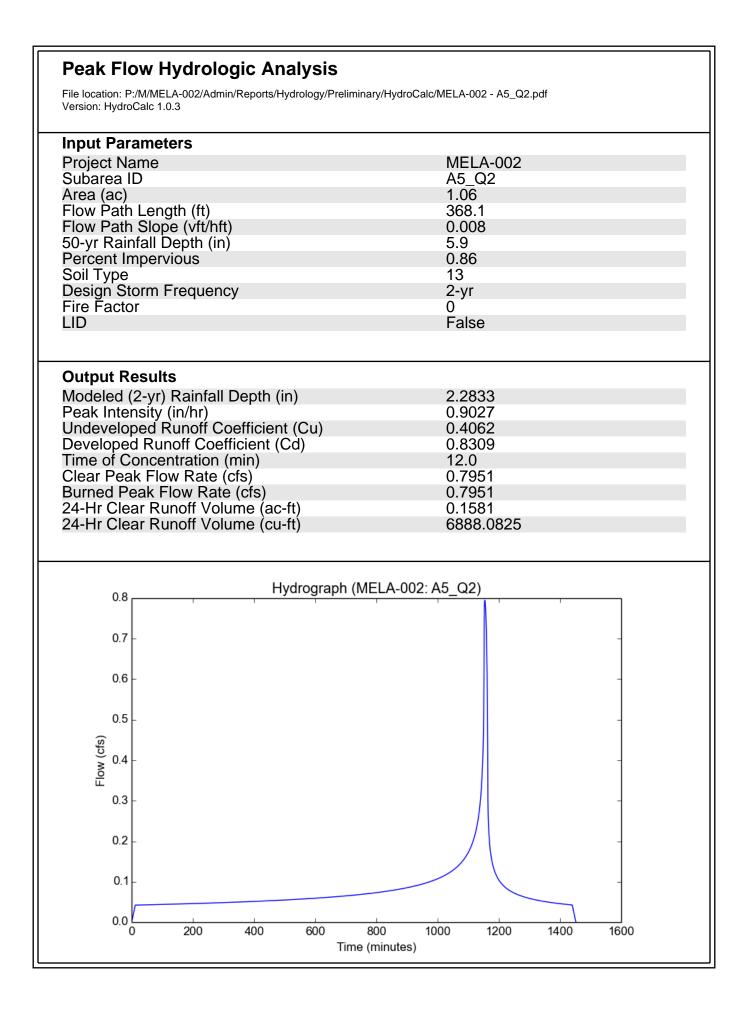


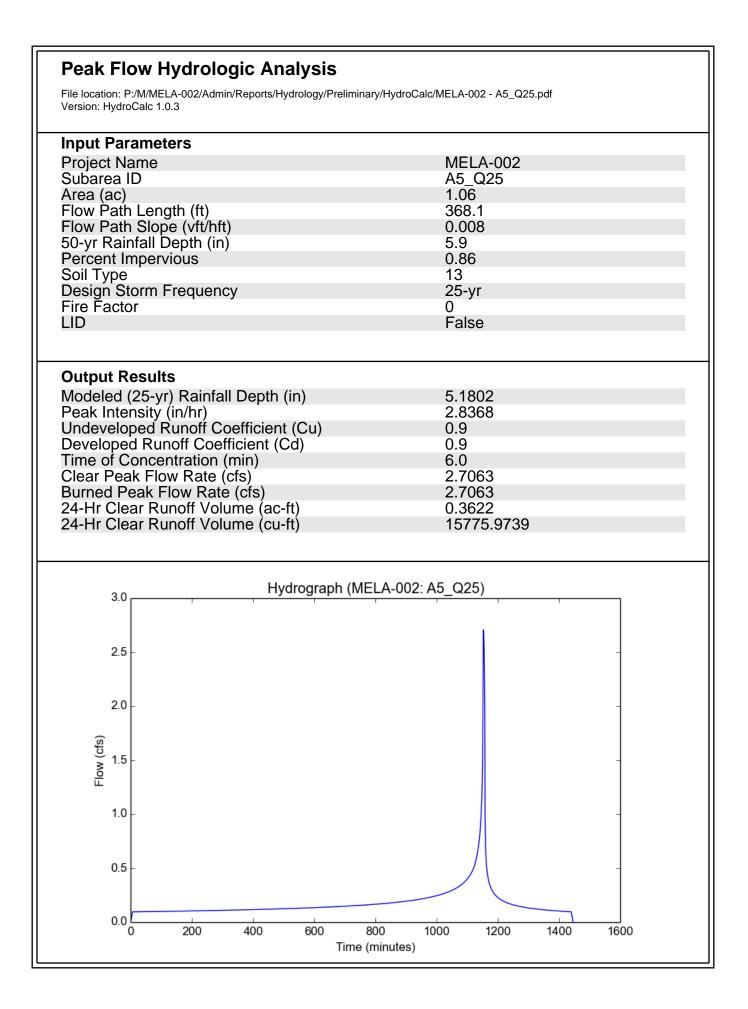


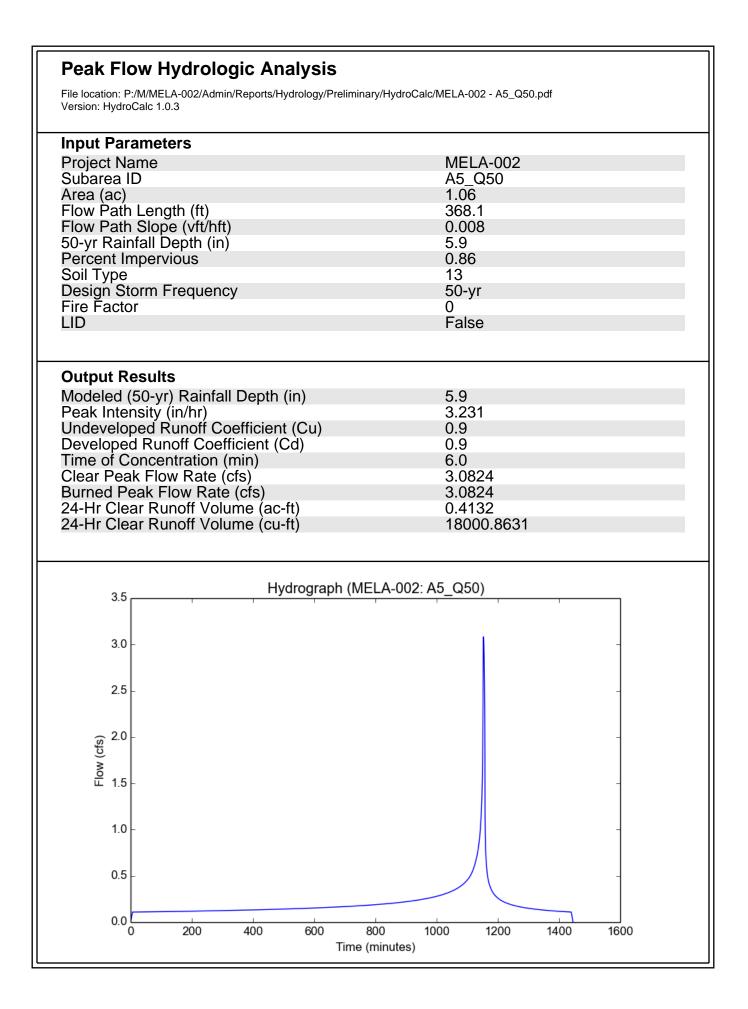


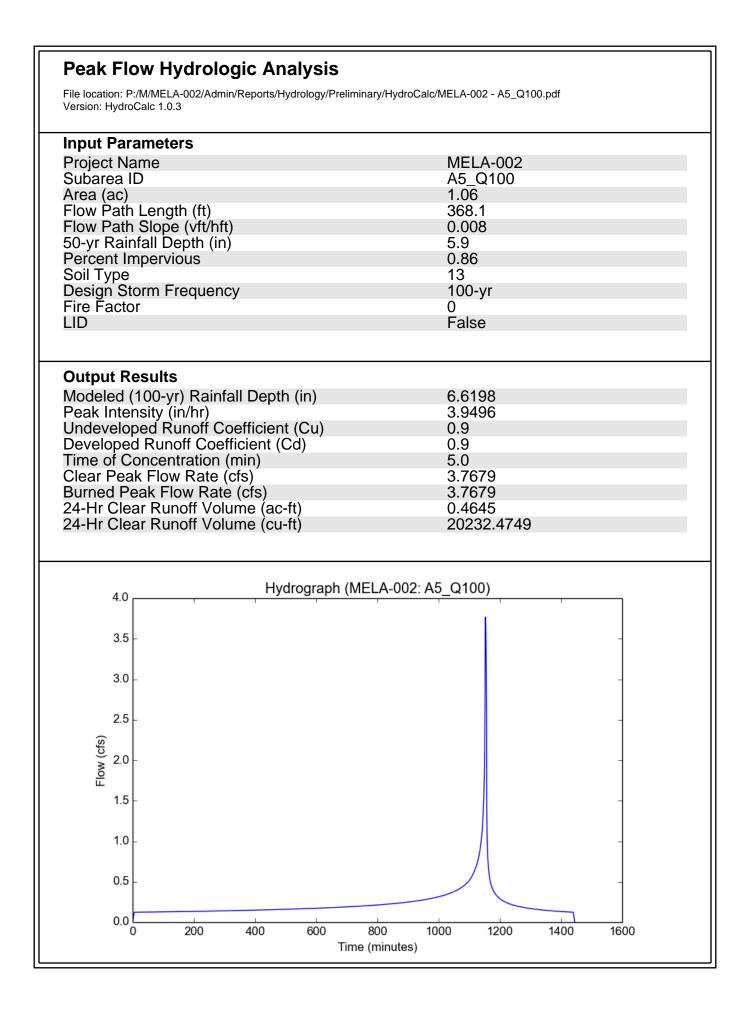


Area A5 - Proposed Conditions Hydrology Calculations (2, 25, 50, 100-year Storm Event)

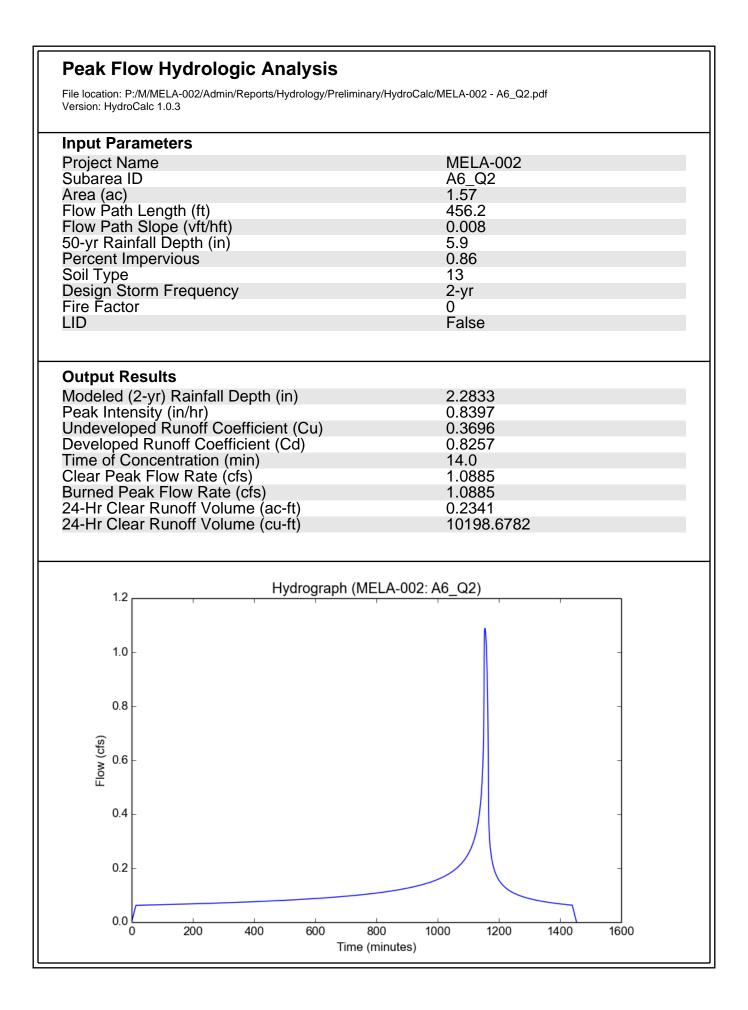


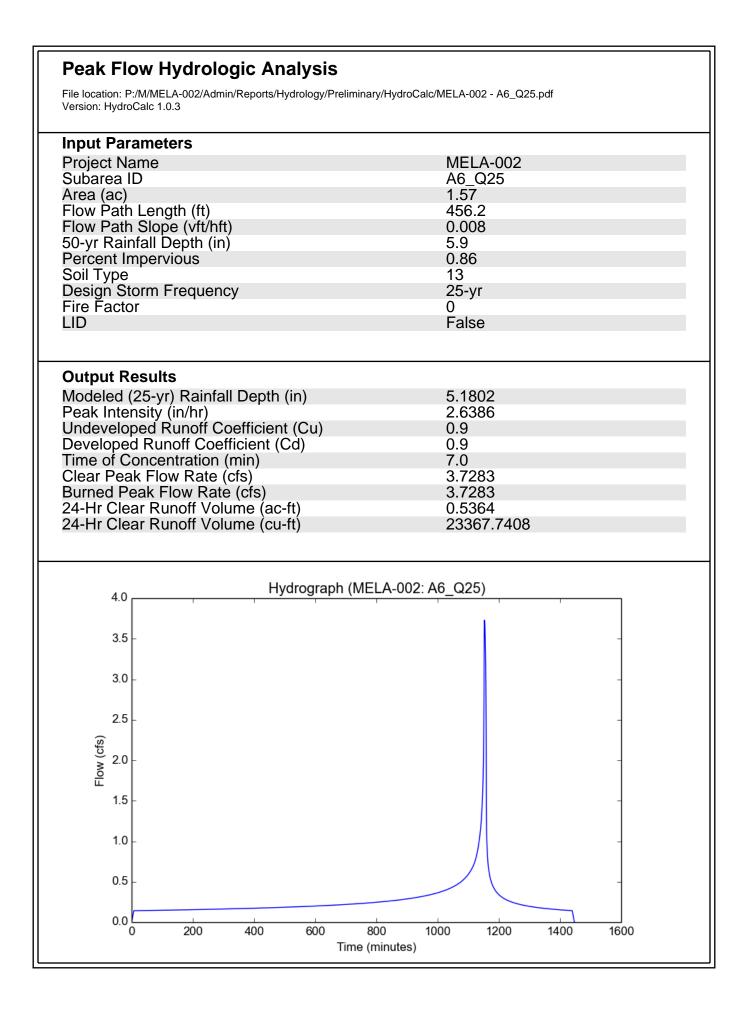


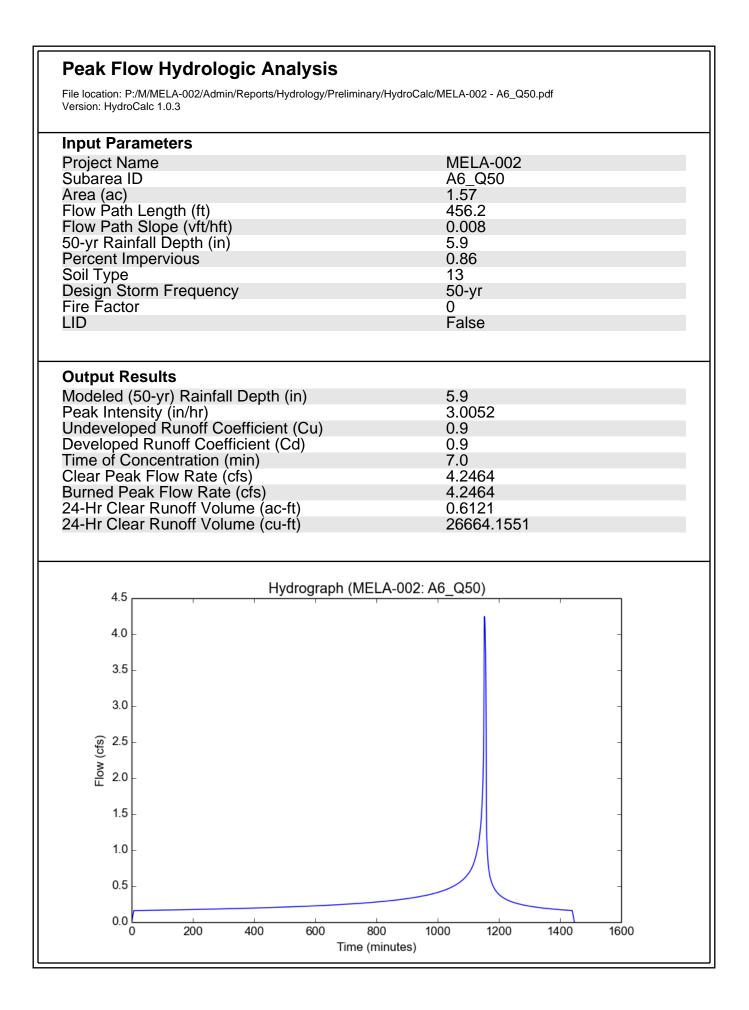


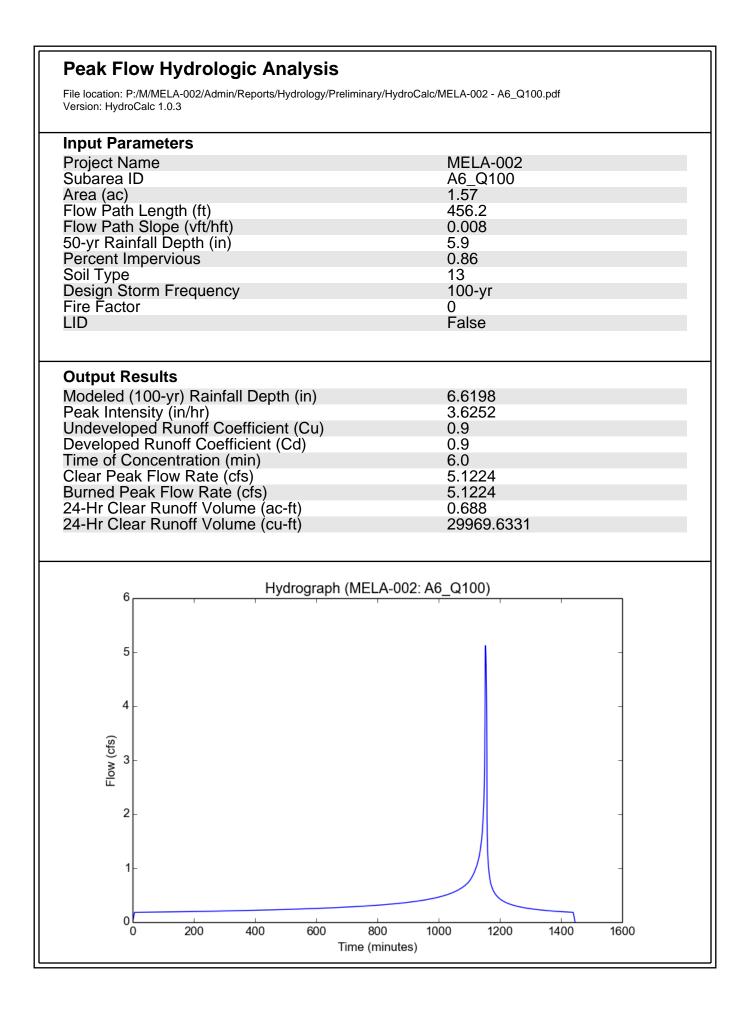


Area A6 - Proposed Conditions Hydrology Calculations (2, 25, 50, 100-year Storm Event)





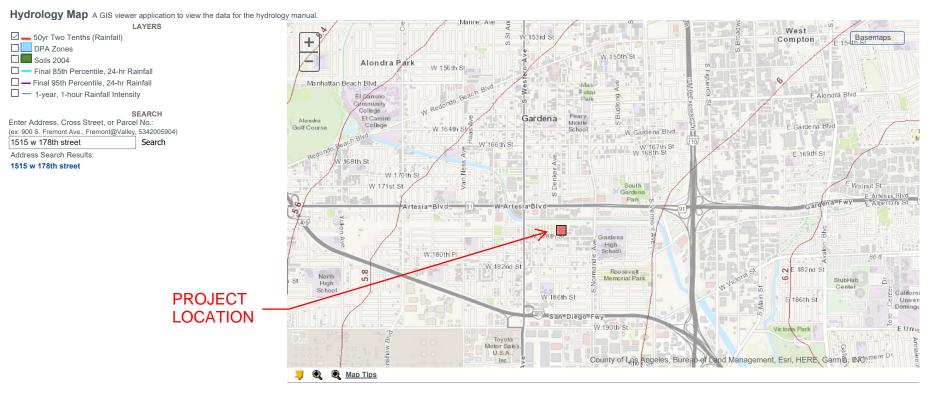




APPENDIX C REFERENCES

50-Year 24 Hr Rainfall Depth

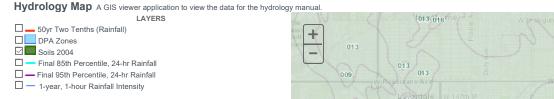
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SOIL TYPE

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009





1515 w 178th street



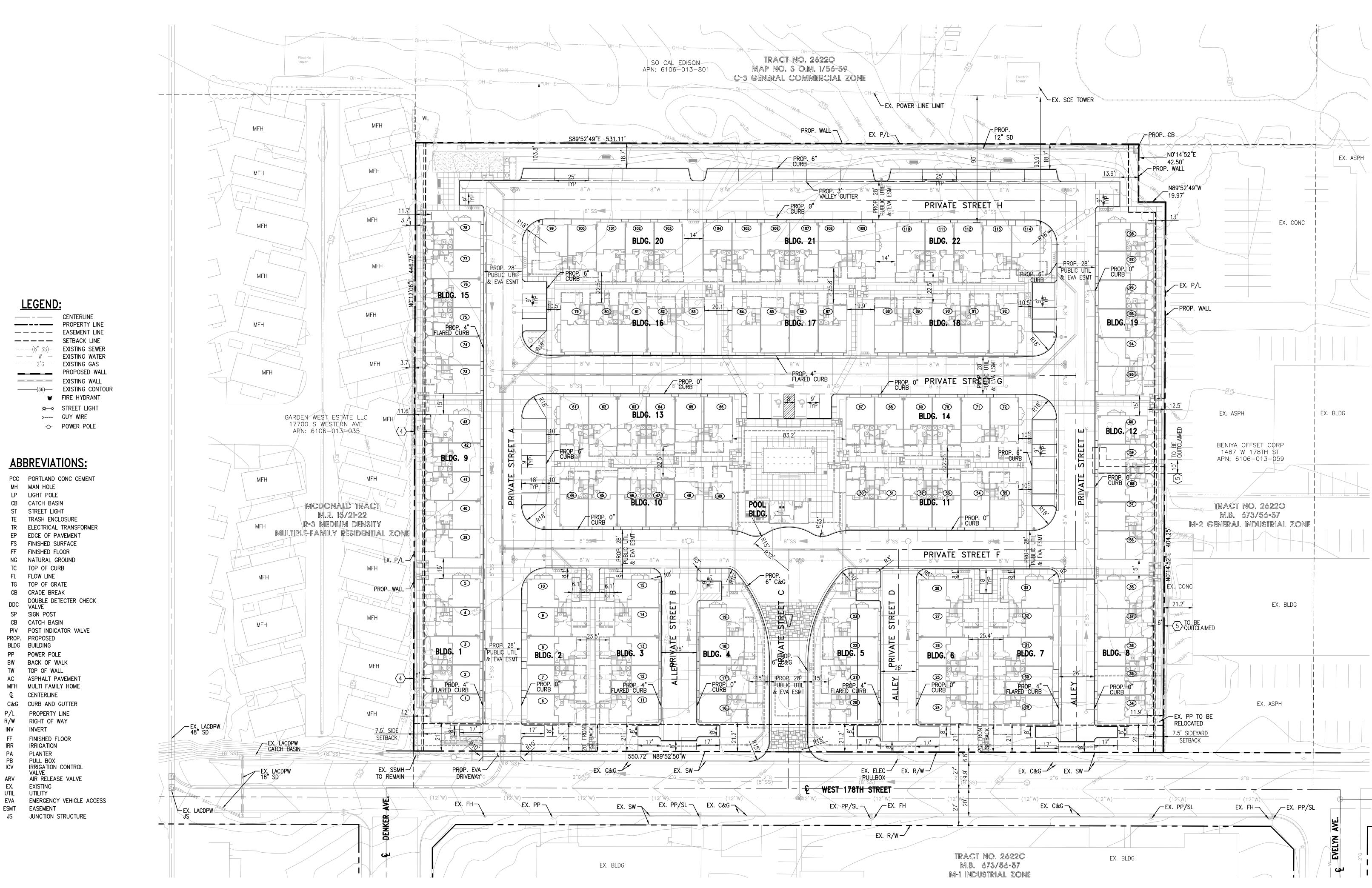
APPENDIX D

Proportion Impervious Data

Proportion Impervious Data

Code	Land Use Description	% Impervious
1111	High-Density Single Family Residential	42
1112	Low-Density Single Family Residential	21
1121	Mixed Multi-Family Residential	74
1122	Duplexes, Triplexes and 2-or 3-Unit Condominiums and Townhouses	55
1123	Low-Rise Apartments, Condominiums, and Townhouses	86
1124	Medium-Rise Apartments and Condominiums	86
1125	High-Rise Apartments and Condominiums	90
1131	Trailer Parks and Mobile Home Courts, High-Density	91
1132	Mobile Home Courts and Subdivisions, Low-Density	42
1140	Mixed Residential	59
1151	Rural Residential, High-Density	15
1152	Rural Residential, Low-Density	10
1211	Low- and Medium-Rise Major Office Use	91
1212	High-Rise Major Office Use	91
1213	Skyscrapers	91
1221	Regional Shopping Center	95
1222	Retail Centers (Non-Strip With Contiguous Interconnected Off-Street	96
1223	Modern Strip Development	96
1224	Older Strip Development	97
1231	Commercial Storage	90
1232	Commercial Recreation	90
1233	Hotels and Motels	96
1234	Attended Pay Public Parking Facilities	91
1241	Government Offices	91
1242	Police and Sheriff Stations	91
1243	Fire Stations	91
1244	Major Medical Health Care Facilities	74
1245	Religious Facilities	82
1246	Other Public Facilities	91
1247	Non-Attended Public Parking Facilities	91
1251	Correctional Facilities	91
1252	Special Care Facilities	74
1253	Other Special Use Facilities	86
1261	Pre-Schools/Day Care Centers	68
1262	Elementary Schools	82
1263	Junior or Intermediate High Schools	82
1264	Senior High Schools	82
1265	Colleges and Universities	47
1266	Trade Schools and Professional Training Facilities	91
1271	Base (Built-up Area)	65
1271.01	Base High-Density Single Family Residential	42
	Base Duplexes, Triplexes and 2-or 3-Unit Condominiums and T	55

APPENDIX D TENTATIVE TRACT MAPS



TITLE INFORMATION:

THE FOLLOWING TITLE INFORMATION WAS DERIVED FROM A PRELIMINARY TITLE REPORT ISSUED BY FIRST AMERICAN TITLE COMPANY, ORDER NO.: 0SA-5570229 (TC) DATED JULY 18, 2018 AT 7:30 A.M.

- $\langle \# \rangle$ denotes plotted item.
- 1–3 TAX ITEMS
- $\langle 4 \rangle$ an easement for sanitary sewer and incidental purposes in the DOCUMENT RECORDED AUGUST 16, 1961 AS BOOK D1323, PAGE 938 OF OFFICIAL RECORDS.
- $\langle 5 \rangle$ an easement for poles and incidental purposes in the document RECORDED FEBRUARY 20, 1962 AS BOOK D1518, PAGE 542 OF OFFICIAL RECORDS.

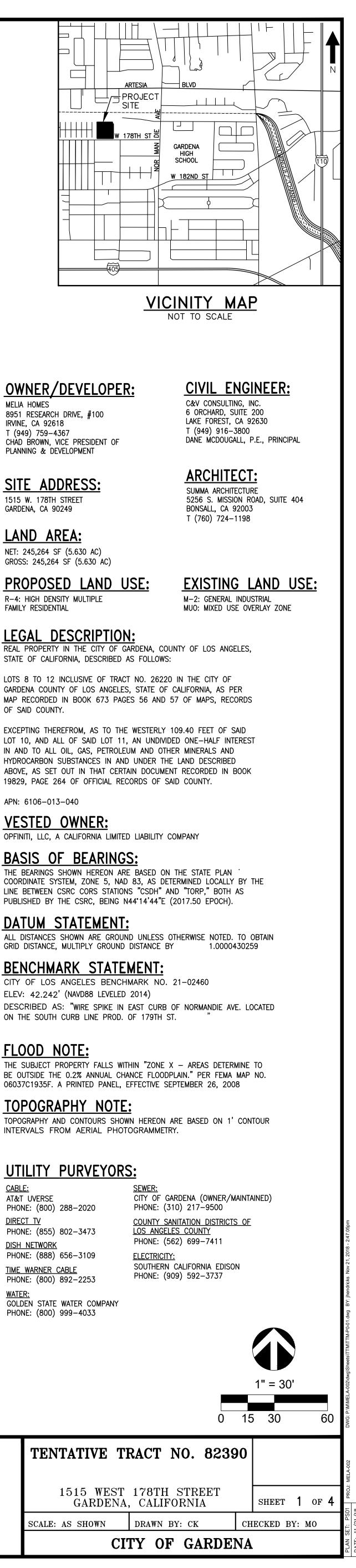
TENTATIVE TRACT NO. 82390

1515 WEST 178th STREET FOR CONDOMINIUM PURPOSES CITY OF GARDENA, COUNTY OF LOS ANGELES STATE OF CALIFORNIA

NOTES:

1. EXISTING ONSITE REMOVALS ARE SHOWN PER SEPARATE FUTURE ONSITE DEMOLITION PLAN.

Γ				REVISIONS			OWNER/DEVELOPER
	NO	DATE	INITIAL	DESCRIPTION	APP	DATE	MH MELIA HO



OWNER/DEVELOPER:

MELIA HOMES 8951 RESEARCH DRIVE, #100 IRVINE, CA 92618 T (949) 759-4367 CHAD BROWN, VICE PRESIDENT OF PLANNING & DEVELOPMENT

SITE ADDRESS: 1515 W. 178TH STREET GARDENA, CA 90249

LAND AREA:

NET: 245,264 SF (5.630 AC) GROSS: 245,264 SF (5.630 AC)

R-4: HIGH DENSITY MULTIPLE FAMILY RESIDENTIAL

LEGAL DESCRIPTION:

OF SAID COUNTY.

APN: 6106-013-040

VESTED OWNER:

BASIS OF BEARINGS:

DATUM STATEMENT:

BENCHMARK STATEMENT:

ELEV: 42.242' (NAVD88 LEVELED 2014) ON THE SOUTH CURB LINE PROD. OF 179TH ST.

FLOOD NOTE:

TOPOGRAPHY NOTE:

UTILITY PURVEYORS:

	<u> </u>
<u>CABLE:</u>	<u>SI</u>
AT&T UVERSE	C
PHONE: (800) 288–2020	P
<u>DIRECT_TV</u>	<u>C</u>
PHONE: (855) 802–3473	L(
<u>DISH NETWORK</u>	P
PHONE: (888) 656–3109	<u>El</u>
<u>TIME WARNER CABLE</u>	S
PHONE: (800) 892–2253	P
<u>WATER:</u> GOLDEN STATE WATER COMPANY	

PHONE: (800) 999-4033



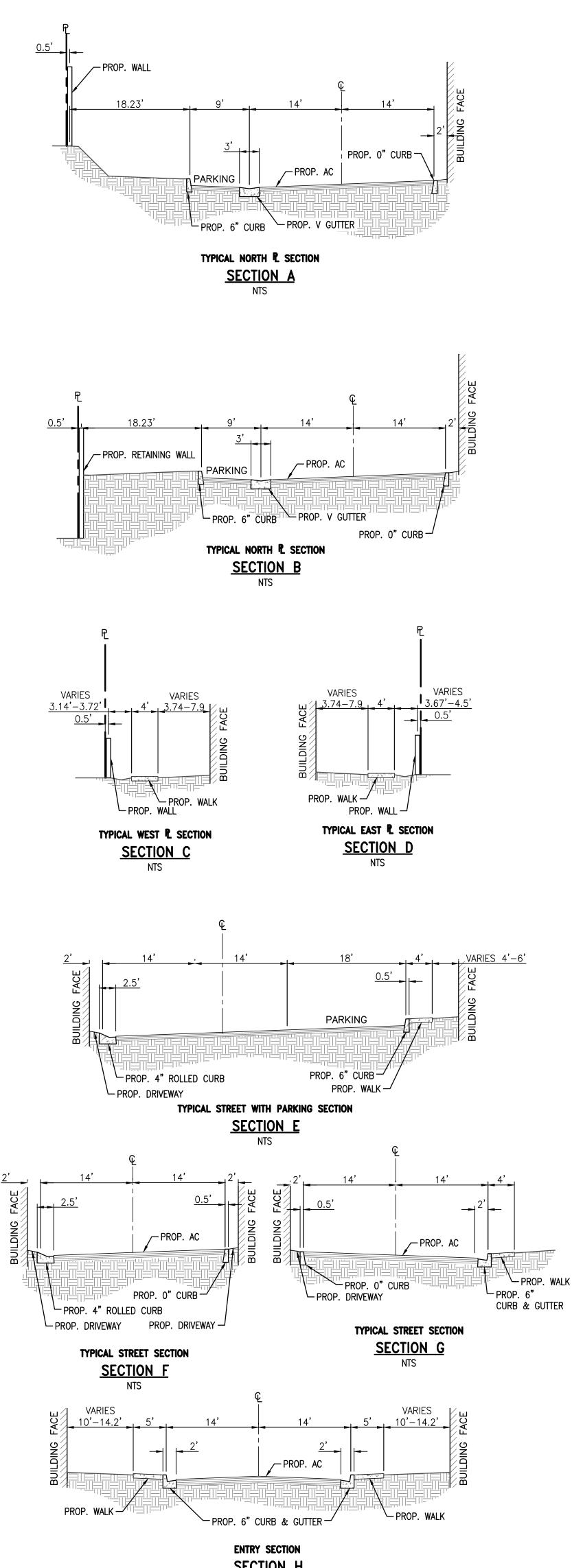
MELIA HOMES 8951 RESEARCH DR. #100 IRVINE, CA 92618 (949) 759-4367

PREPARED BY :

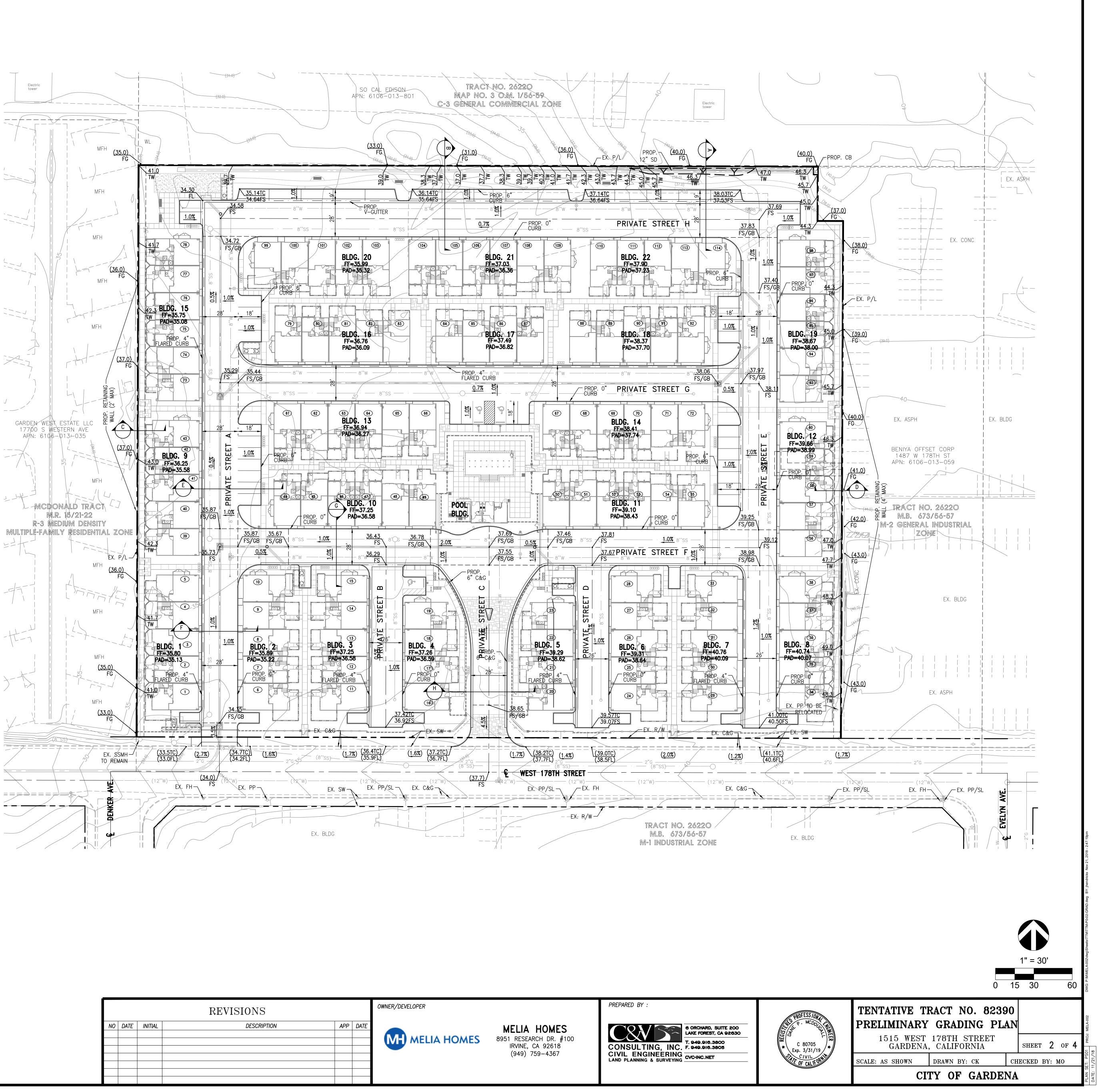
6 ORCHARD, SUITE 200 LAKE FOREST, CA 92630 T. 949.916.3800 CONSULTING, INC, F. 949.916.3805 CIVIL ENGINEERING LAND PLANNING & SURVEYING



SCALE: AS SHOWN

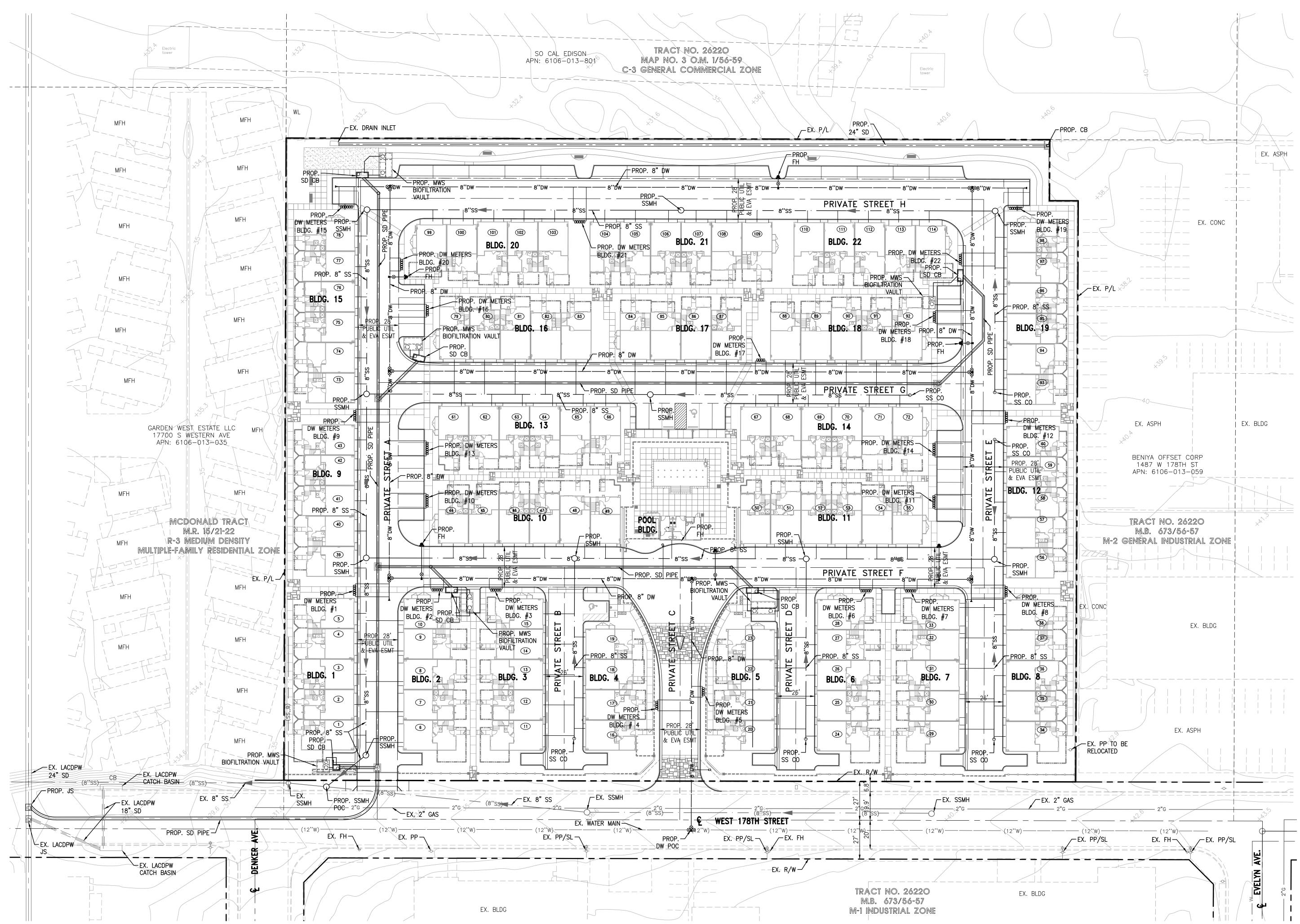


SECTION H

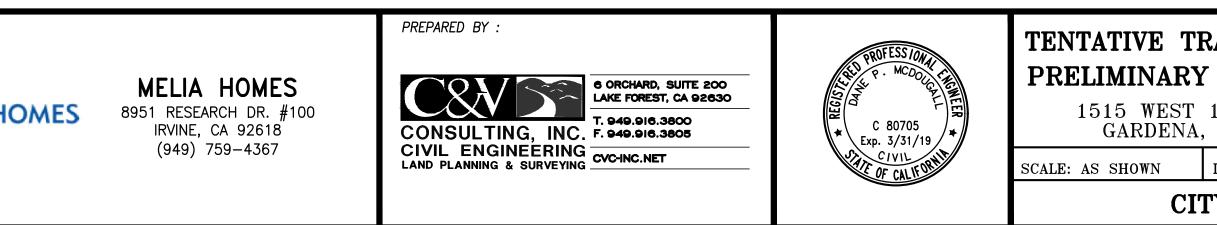


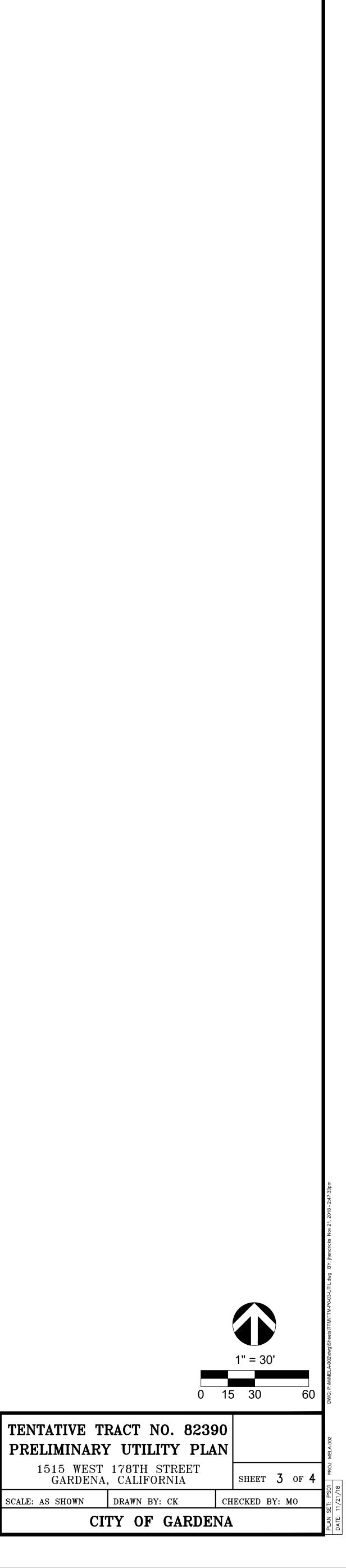
REVISIONS						OWNER/DEVELOPER
NO	DATE	INITIAL	DESCRIPTION	APP	DATE	
						MH MELIA H





			REVISIONS			OWNER/DEVELOPER
NO	DATE	INITIAL	DESCRIPTION	APP	DATE	
						MH MELIA H





NOTES:

- 1. FIRE DEPARTMENT VEHICULAR ACCESS ROADS SHALL HARDSCAPE ALL WEATHER ACCESS IN ACCORDANCE WITH THE DEPARTMENT'S ALL WEATHER ACCESS REQUIREMENTS. FIRE CODE 503.2.3
- 2. FIRE DEPARTMENT VEHICULAR ACCESS ROADS MUST BE INSTALLED AND MAINTAINED IN A SERVICEABLE MANNER PRIOR TO AND DURING THE TIME OF CONSTRUCTION. FIRE CODE 501.4
- 3. APPROVED BUILDING ADDRESS NUMBERS, BUILDING NUMBERS OR APPROVED BUILDING IDENTIFICATION SHALL BE PROVIDED AND MAINTAINED SO AS TO BE PLAINLY VISIBLE AND LEGIBLE FROM THE STREET FRONTING THE PROPERTY. THE NUMBERS SHALL CONTRAST WITH THEIR BACKGROUND, BE ARABIC NUMERALS OR ALPHABET LETTERS, AND BE A MINIMUM OF 4 INCHES HIGH WITH A MINIMUM STROKE WIDTH OF 0.5 INCH. FIRE CODE 505.1
- 4. THE REQUIRED FIRE FLOW FOR FIRE HYDRANTS AT THIS LOCATION IS 2000 GPM. AT 20 PSI RESIDUAL PRESSURE, FOR A DURATION OF 2 HOURS OVER AND ABOVE MAXIMUM DAILY DOMESTIC DEMAND. FIRE CODE 507.3
- 5. ALL FIRE HYDRANTS SHALL MEASURE 6"X4"X2-1/2", BRASS OR BRONZE, CONFORMING TO AMERICAN WATER WORKS ASSOCIATION STANDARD C503. OR APPROVED EQUAL, AND SHALL BE INSTALLED IN COMPLIANCE WITH FIRE CODE 507.5.
- 6. ALL REQUIRED PUBLIC FIRE HYDRANTS SHALL BE INSTALLED, TESTED AND ACCEPTED PRIOR TO BEGINNING CONSTRUCTION. FIRE CODE 501.4.
- 7. THE REQUIRED FIRE FLOW FOR A SINGLE PRIVATE ON-SITE FIRE HYDRANT AT THIS LOCATION IS 1,250 GPM AT 20 PSI RESIDUAL PRESSURE. IF MORE THAN ONE ON-SITE FIRE HYDRANT IS REQUIRED, THE ON-SITE FIRE FLOW SHALL BE THE SAME AS REQUIRED FOR PUBLIC HYDRANTS IN ACCORDANCE WITH APPENDIX TABLE B1-5.1. FIRE CODE C106.
- 8. ALL ON-SITE FIRE HYDRANT SHALL BE INSTALLED, TESTED AND APPROVED PRIOR TO BUILDING OCCUPANCY. FIRE CODE 901.5.1

PROJECT GENERAL NOTES:

- 1. ALL FIRE ACCESS LANES MEET LACFD MINIMUM REQUIREMENTS 19' & 45' RADII.
- 2. THIS PROJECT DOES NOT HAVE ANY FUEL MODIFICATION OR WILD LAND EXPOSURES AND IS NOT IN A "VERY HIGH FIRE HAZARD" ZONE.
- 3. THIS PROJECT IS DESIGNED IN CONFORMANCE WITH THE CBC, 2015 EDITION.
- ALL FIRE ACCESS ROADS SHALL BE ALL WEATHER, MEET THE CRITERIA OF AN ALL WEATHER DRIVING SURFACE AND COMPLY WITH LACFD GUIDELINE FOR FIRE APPARATUS ROADS.
- 5. LARGEST BUILDING FOOTPRINT SQ. FOOTAGE =10,798 SF
- 6. BUILDINGS ARE DESIGNATED TYPE-VB
- 7. BUILDING OCCUPANCY IS DESIGNATED AS R2-TOWNHOMES.
- 8. ALL BUILDINGS ON THE SITE WILL BE SPRINKLERED PER NFPA 13D.
- **ALL BUILDING INFORMATION AND SITE PLAN TO BE CONFIRMED WITH ARCHITECTURAL LAYOUT

IRRIGATION CONTROL

AIR RELEASE VALVE

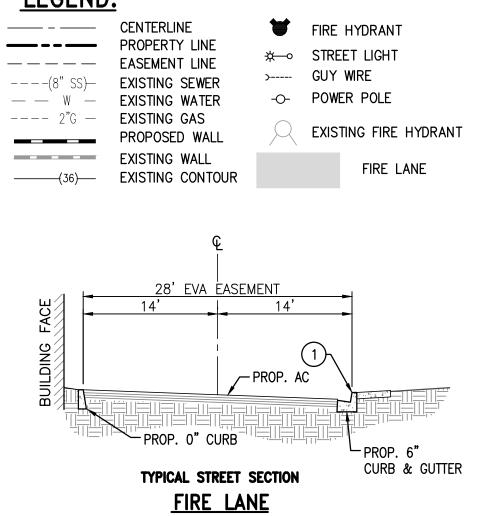
EMERGENCY VEHICLE ACCESS

ABBREVIATIONS:

PCC MH LP CB ST TE TR EP FS FF NG TC FL GB DDC SP CB	PORTLAND CONC CEMENT MAN HOLE LIGHT POLE CATCH BASIN STREET LIGHT TRASH ENCLOSURE ELECTRICAL TRANSFORMER EDGE OF PAVEMENT FINISHED SURFACE FINISHED FLOOR NATURAL GROUND TOP OF CURB FLOW LINE TOP OF GRATE GRADE BREAK DOUBLE DETECTER CHECK VALVE SIGN POST CATCH BASIN POST INDICATOR VALVE	PP BW TW AC MFH & C&G P/L R/W INV FF IRR PA PB ICV ARV EX. UTIL	POWER POLE BACK OF WALK TOP OF WALL ASPHALT PAVEMENT MULTI FAMILY HOME CENTERLINE CURB AND GUTTER PROPERTY LINE RIGHT OF WAY INVERT FINISHED FLOOR IRRIGATION PLANTER PULL BOX IRRIGATION CONTRO VALVE AIR RELEASE VALV EXISTING UTILITY
			UTILITY
PIV PROP.	POST INDICATOR VALVE PROPOSED	EVA ESMT	EMERGENCY VEHICL EASEMENT

LEGEND

BLDG BUILDING



NTS

(FIRE LANE)

NO PARKING

VIOLATING VEHICLES WILL BE CITED OR TOW

AT OWNER'S EXPENS

CVC 22658A CVC 22500.1 GARDENA POLICE DEPT. (310) 217-9600

18"

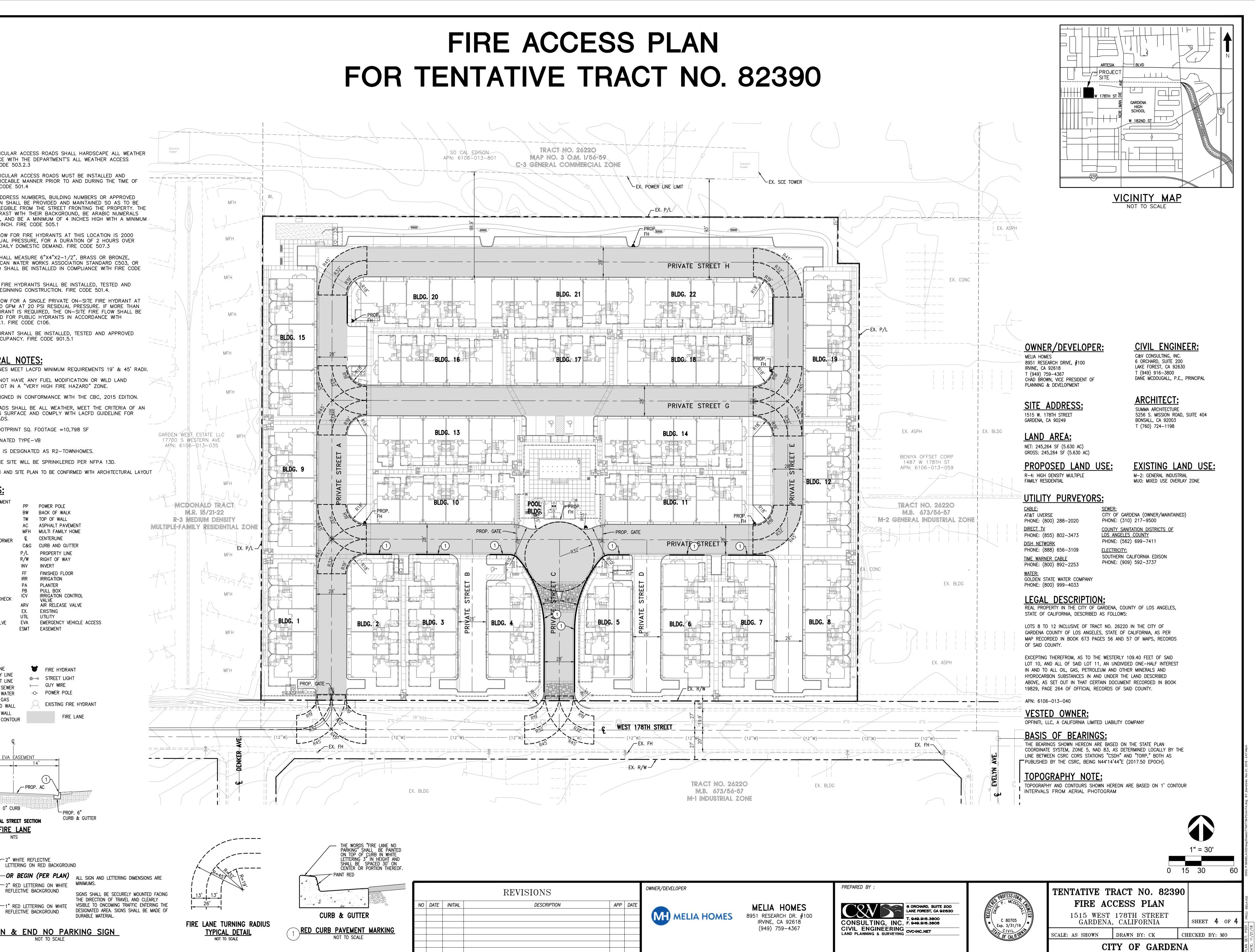
END -

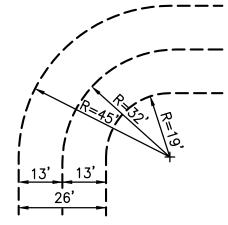
~2" WHITE REFLECTIVE

LETTERING ON RED BACKGROUND

REFLECTIVE BACKGROUND

REFLECTIVE BACKGROUND











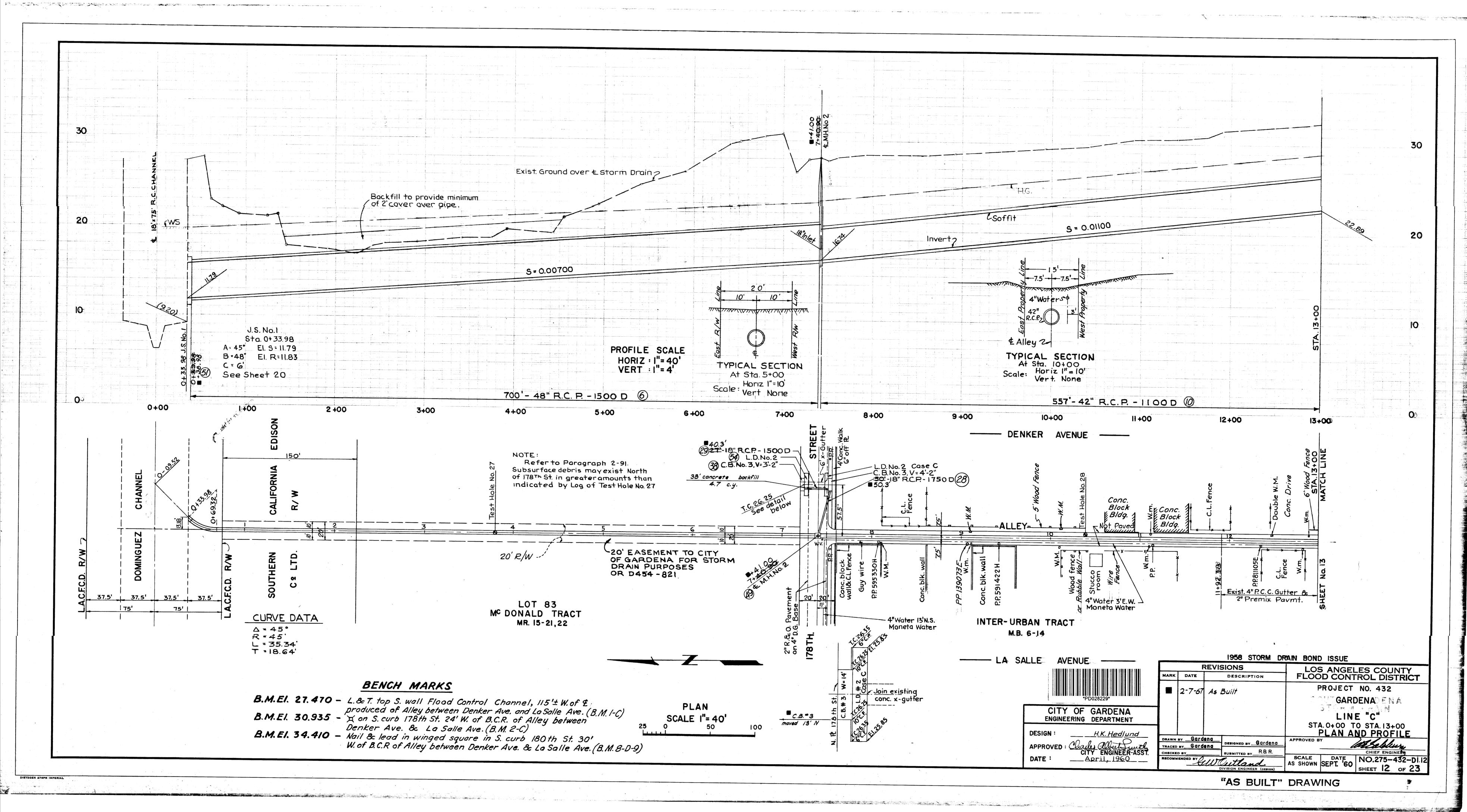
NOT TO SCALE

DURABLE MATERIAL.

- 2" RED LETTERING ON WHITE MINIMUMS.



APPENDIX E AS-BUILT



x-gutfer	*PD028229*		GA
	CITY OF GARDENA ENGINEERING DEPARTMENT		LI
-	DESIGN: <u>H.K. Hedlund</u> APPROVED: <u>Charles Albert Suith</u> CITY ENGINEER-ASST. DATE: <u>April, 1960</u>	DRAWN BY <u>Gardena</u> TRACED BY <u>Gardena</u> CHECKED BY <u>Gardena</u> RECOMMENDED BY <u>R.B.R.</u> SUBMITTED BY <u>R.B.R.</u>	STA. 0+00 PLAN APPROVED BY SCALE DATE AS SHOWN SEPT. '6
		TIVISION ENGINEER (DESIGN)	DRAWING